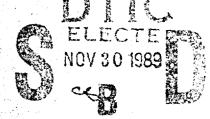
ANEME SON

PISCO AND LA JOYA AIR BASES, PERU



NOVEMBER 1989

DESTRUCTION STATEMENT A

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AIR FORCE ENGINEERING and SERVILES CENTER

TYNDALL AIR FORCE BASE,

FLORIDA 32403-6001



AIRFIELD PAVEMENT EVALUATION

OF

PERUVIAN AIR BASES

PREPARED FOR
TACTICAL AIR COMMAND (TAC)

ΒY

HQ AFESC PAVEMENT EVALUATION TEAM

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FLORIDA 32403-6001

PUBLISHED NOVEMBER 1989

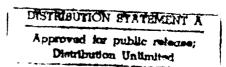


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EXECUTIVE SUMMARY

1. At the request of Tactical Air Command, a Pavement Evaluation Team from HQ Air Force Engineering and Services Center (AFESC) performed modified destructive airfield pavement evaluations at LaJoya Air Base and Pisco Air Base, Peru during 26 April-10 May 1989. The purposes were to establish physical property data, determine pavement load-carrying capabilities, and identify any existing or potential pavement distresses.

2. LAJOYA AIR BASE

- a. The LaJoya airfield consists primarily of 13,154 ft long runway and a parallel taxiway which is also used as a runway. Primary apron features include the East and West Aprons with aircraft shelters dispersed around each. Runway 17/35 and the Parallel Taxiway are essentially 3-layered flexible pavement systems. Tests were conducted every 1000 feet on the taxiway and runway to define the soil strength profiles. Tests were also conducted in random spots throughout the two major parking aprons.
- b. Pavement conditions at LaJoya range from FAIR to VERY GOOD condition. The portland cement concrete (PCC) parking aprons are generally FAIR and the asphaltic concrete (AC) runway and taxiway are in GOOD and VERY GOOD condition, respectively. Joint sealant is virtually non-existent throughout all PCC features. This has led to edge spalls that present a FOD hazard. Few distresses exist in the AC pavements. The underlying soils are unusually strong which is key to the overall pavement strength. No significant load limitations exist on this airfield.

3. PISCO AIR BASE

a. The Pisco airfield consists primarily of a 10,000 ft long runway and a parallel taxiway. The parallel taxiway adjoins the runway via 5 ladder taxiways. One other flexible pavement taxiway is adjacent to the PCC parking apron. All flexible pavements are three-layer systems.

- b. Pavement conditions at Pisco range from FAILED to EXCELLENT. The PCC parking apron is generally VERY POOR and the AC runway is VERY GOOD to EXCELLENT. The remaining PCC and AC taxiways vary in condition. There are no indications of structural distress on the runway. Joint sealant is virtually non-existent throughout all PCC features. This, too, has led to edge spalls that present a FOD hazard.
- c. Shattered slabs, indicative of pavement failure, are common throughout the Parking Apron. Significant load limitations should be imposed on the PCC parking apron. The weakest pavements are sections of the Parallel Taxiway (Feature TO4A) and part of the Main Parking Apron (Features AO1B and AO2B). Catastrophic failure is unlikely, however, the existing slabs are in POOR condition, or worse, on many of the features. Recommend the severely distressed sections be replaced.

SECTION I: INTRODUCTION

A. SCOPE

A Headquarters Air Force Engineering and Services Center (HQ AFESC) Pavement Evaluation Team (PET) performed modified destructive airfield pavement evaluations at LaJoya Air Base and Pisco Air Base, Peru, at the request of Headquarters, Tactical Air Command (TAC). Field testing was accomplished during 26 April-10 May 1989. The purposes of the evaluations were to investigate distress patterns on the airfields, establish physical property data, determine the in situ properties of the pavement structures for calculating allowable gross loads (AGLs), and identify reasons for existing or potential pavement distress.

This report is intended as an aid to individuals, organizations, and agencies. With this in mind, the narrative is brief but is supplemented by many detailed appendices. LaJoya pavement evaluation is reported first in each section, followed by the Pisco evaluation. A list of the included appendices is provided below.

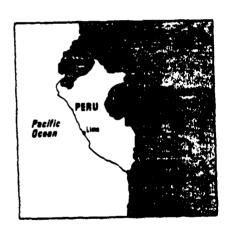
Appendix Description

- A <u>Airfield Layout Plan</u>: This plan graphically depicts different pavement features of the airfield.
- B This appendix not used.
- C Test Location and Core Location Plans:
 These plans document the locations where tests were conducted and cores were extracted. Core thicknesses and flexural strengths are also recorded on the core location plan.
- D <u>Condition Survey</u>: This plan shows the operating condition of the airfield pavements. The condition ratings are a qualitative assessment of the pavement surface conditions based upon visual observations and engineering judgment.
- E <u>Summary of Physical Property Data</u>: Physical properties of each pavement feature are tabulated. Included are feature dimensions, material types, thicknesses of layers, and engineering properties.

- F Allowable Gross Loads (AGLs): A listing of the allowable magnitude of loads at four pass intensity levels for each aircraft group.
- Related Information: Included in this are
 Aircraft Group Indices, Gross Weight Limits for
 Aircraft Groups, Pass Intensity Levels,
 Climatological Chart, and Climatological
 Narrative.

B. SITE LOCATIONS

LaJoya Air Base is located in southwestern Peru, near the city of Arequipa. It lies in desert terrain, where there is little precipitation. Pisco is located approximately 150 miles south of Lima and located on the Pacific coast. Respective locations are shown on the map below.





1. LaJoya

2. Pisco

SECTION II. EVALUATION PROCEDURES

A. FIELD TESTING

Pavement testing was done by extracting pavement cores and conducting Small Aperture Tests (SAT) and Dynamic Cone Penetrometer (DCP) tests in the pavement core holes. SAT is a modified California Bearing Ratio (CBR) test used to determine the strength of supporting soils. The DCP measures penetration resistance correlated to CBRs for the supporting soils. For evaluation of Portland cement concrete (PCC), corresponding CBRs were correlated to moduli of subgrade reaction (k-values) used in design and evaluation of rigid pavements. Additionally, pavement cores, along with soil samples from both bases, were shipped to Tyndall AFB for material testing.

B. CONDITION RATINGS

Pavement condition definitions range from EXCELLENT (like new) to FAILED (unsafe for aircraft traffic). Condition ratings are a qualitative assessment of the pavement surface and should not be confused with the structural capacity of a pavement. For example, a pavement surface may rate EXCELLENT, but have underlying pavement or soil conditions that could result in pavement failure under the applied load of a given aircraft. On the other hand, a pavement may be structurally sound but the surface condition may be hazardous for aircraft traffic.

C. LABORATORY TESTING

Pavement core samples were returned to Tyndall AFB for laboratory testing. PCC cores were tested for strength by tensile splitting in accordance with ASTM's "Standard Test Methods." The six-inch diameter core tensile splitting strengths were then converted to flexural strengths by using an empirical relationship (Ref 4). Flexural strengths are reported on the "Core Location Plan" (Appendix C) and in Appendix E.

D. MATERIAL PROPERTIES

The load-carrying capacities of the pavements reported herein are based on material properties representative of the in-place conditions at the time this field investigation was conducted. Exact agreement between behavior of the facilities as shown by this evaluation and that which may actually occur under traffic cannot be expected, primarily because of the difficulties of determining the exact traffic that produces the behavior, and also because material properties change due to environmental factors such as precipitation, freeze-thaw cycles, and age. These changes must be considered in future planning, especially where a change in mission will result in an increase in aircraft loads and/or aircraft traffic volume.

E. CLIMATIC DATA

Appendix G provides a summary of climatic data for both airfields.

SECTION III: METHODOLOGY OF ANALYSIS

A. PHYSICAL PROPERTY DATA

The parameters used for this evaluation are summarized in Appendix E. The data presented were selected as the most representative strength values for each feature. Strength of flexible pavements (asphaltic concrete, AC) are based on the the conventional CBR method of design and evaluation. Each unique soil layer was tested to determine the CBR of the layer. CBRs were also measured on the rigid pavement (Portland cement concrete, PCC) supporting soils, and then correlated to moduli of subgrade reaction, or k-value. Rigid pavements were then evaluated based on the Westergaard theory of design and evaluation.

B. DETERMINATION OF ALLOWABLE GROSS LOADS

The AGLs were determined by a computer program based on procedures in AFM 88-24 and AFR 93-5. The AGL for a feature was reduced 25 percent if the condition rating for the feature was POOR or worse. Appendix E outlines the engineering properties used to calculate the AGLs.

Failure criteria used in the allowable load analysis is different for rigid and flexible pavements. Rigid (and composite) pavement failure criteria is partly based on a limiting tensile stress of the concrete. Conversely, compressive subgrade strain is one failure parameter used in the AGL calculation of flexible pavement systems.

C. EXAMPLE PROBLEM

The following example (employing data from this report) illustrates how to use the allowable gross load information.

<u>Problem</u>: The Peruvian Air Force wants to know how many times a 550-kip (1 kip = 1000 pounds) C-5 aircraft can traffic on Feature T01A of the Pisco airfield. How many C-5 passes can be supported before the pavement fails?

<u>Solution</u>: From Appendix F, the Allowable Gross Loads for a C-5 at Pass Intensity Levels I-IV (50,000, 15,000, 3,000, and 500) are 507, 513, 536, and 581 kips, respectively. The weights and passes are plotted on semi-log paper as shown in Figure 1. The completed graph indicates a 550-kip C-5 can make approximately 1,500 passes on Feature T01A before the pavement fails.

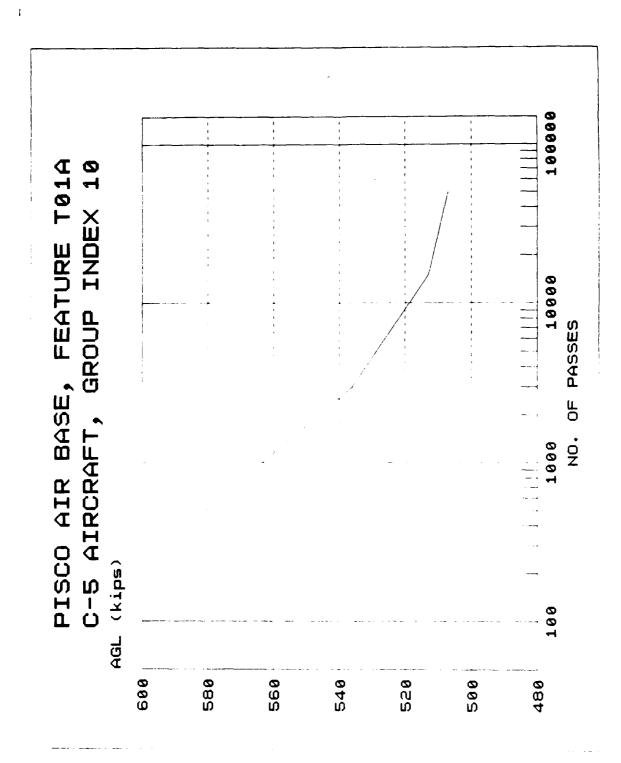


FIGURE 1

D. PAVEMENT CLASSIFICATION NUMBER

The International Civil Aviation Organization (ICAO) has developed and adopted a standardized method of reporting pavement strength. This procedure is known as the Aircraft Classification Number/Pavement Classification Number (ACN/PCN) method (Reference 3). In support of this international system, PCNs are provided for each pavement feature on the different airfields. PCNs were calculated based on Group 9 aircraft at Pass Intensity Level I (50,000 passes). PCNs for respective airfields are listed in Appendix F. A brief explanation on the PCN code is shown below for PCN = 31/R/A/W/T.

PCN FIVE-PART CODE

PCN	Pavement Type	Subgrade Strength	Tire Pressure	Method of PCN Determination
Numeric Value	R - Rigid	A	W	T - Technical Evaluation
		В	X	
= 31	F - Flexib	le C	Y	U - Using
		D	Z	Aircraft

EXPLANATION OF TERMS:

Subgrade Strength Codes

Code	Category	Flexible Pavement CBR, %	Rigid Pavement k, pci
A	High	Over 13	Over 400
В	Medium	9 - 13	201-400
С	Low	4 - 8	100-200
D	Ultralow	4	< 100

Tire Pressure Codes

Code	Category	Tire Pressure, psi
W	High	No Limit
X	Medium	146 - 217
Y	Low	74 - 145
Z	Ultralow	0 - 73

SECTION IV. PAVEMENT ASSESSMENT

A. LAJOYA AIR BASE

The LaJoya airfield consists primarily of 13,154 ft long runway and a parallel taxiway which is also used as a runway. Primary apron features include the East and West Aprons with aircraft shelters dispersed around each. Runway 17/35 and the Parallel Taxiway are essentially 3-layered flexible pavement systems. SATs were conducted every 1000 feet on the taxiway and runway on both the base course and subgrade, where possible. This was done to define the soil strength profile. SATs were also conducted in random spots throughout the two major parking aprons.

Pavement conditions at LaJoya range from FAIR to VERY GOOD condition. The PCC parking aprons are generally FAIR and the AC runway and taxiway are in GOOD and VERY GOOD condition, respectively. Joint sealant is virtually non-existent throughout all PCC features. This has led to edge spalls that present a FOD hazard. Few distresses exist in the AC pavements. Specific conditions and recommendations are addressed in the following paragraphs.

1. Runway 17/35

Most of Runway 17/35 is a three-layer flexible pavement system consisting of approximately 6 inches of AC on 18 inches of base course covering the subgrade material. Distinction between the two soil layers was difficult at points, but enough tests were conducted to differentiate between the layers. The first 1600 feet of the 17 end is approximately 12 inches of Portland cement concrete (PCC) placed on 12 inches (design thickness) of base over the subgrade material.

The strength of each layer was determined throughout the runway. The base course strengths consistently tested well above 100% CBR. The subgrade strengths also tested considerably high (30%-75%). This can be attributed to the type of soil and arid climate. The soil is a silty sand that is naturally cemented. Runway soil strength indicate the pavements are adequate to maintain current operations, and more. Specific load carrying capabilities are outlined in Allowable Gross Load tables, Appendix F.

Flexible runway pavements are generally in GOOD-to-VERY GOOD condition. Original construction was four inches of AC followed by a 2-inch overlay to smooth the surface. An asphalt seal coat was subsequently applied to the runway. There are no indications of structural distress. However, several patches have been randomly placed throughout.

The patches were constructed and then sealed with a rich asphalt sealant. Because of the rich sealant, surface shear failures are evident from aircraft and vehicular traffic. This can be expected to occur under sharp turning wheels.

Approximately 6000 feet from the 35 end are 1/4"-1/2" cracks running diagonally across the runway. The cracks do not follow a typical "load-related" pattern. The cracks appear to be caused from shifting of subsurface soils. Earth tremors have been known to occur in the area, and testing geological conditions is beyond the scope of this evaluation. Recommend the cracks be sealed and any further deterioration be monitored.

The PCC at the 17 end is in FAIR condition. Typical distresses include longitudinal and transverse cracks that resulted from extreme slab dimensions. Since the cracks first appeared, aircraft traffic has aggravated the condition. Some of the cracks were chipped to a "V-shape" and filled with concrete. The concrete has since broken in many areas and presents a potential for FOD. A suggested method of repair is to sawcut a minimum of three inches deep on each side of the crack, and remove the distressed concrete to sound material. New concrete should then be placed in the prepared area. The cracks will reappear, in time. However, the new material can be sawcut and sealed to "establish" and control the cracking like a joint. Recommend the the new joints be sealed with a flexible, asphalt-based sealant. Recommend the remaining joints and cracks be cleaned and sealed.

2. Taxiways:

There are six AC ladder taxiways that connect the Parallel Taxiway to the runway. Like the runway, the taxiways' soil strength tested very high. The same type of diagonal cracks that appear on the runway also appear on the Parallel Taxiway at the same location. Withstanding these cracks, the parallel taxiway, and the ladder taxiways are generally in GOOD-to-VERY GOOD condition.

The only obvious distresses on the Parallel Taxiway are some environmentally-related block cracks limited to one paving lane between ladder Taxiways 4 and 5. The only other distresses are the same type of diagonal cracks that were present on the runway. These cracks appear to follow the same pattern and continue along the same geological disturbance path. The recommended maintenance for this type of cracking is to seal the cracks and watch for any further deterioration.

3. Aprons:

The primary aprons consist of the East and West parking ramps which are constructed of 11 to 12 inches of PCC. Typical distresses are primarily intersecting slab cracks and joint spalls.

The intersecting cracks are present in a majority of the slabs. The cracks initially appeared because the slab dimensions are too great. Existing slabs are approximately 25 ft x 25 ft, hence many of the slabs have broken into four pieces. The resulting cracks are not sealed and have subsequently spalled. Recommend the cracks be cleaned and sealed to retard any further deterioration.

The second primary problem throughout the aprons is the joint spalling. Joint sealant is virtually non-existent, which has allowed a passageway for incompressibles to enter the pavement joints. These incompressibles restrict movement when concrete expands, thus resulting in spalled edges. Recommend the spalled edges be sawcut a minimum of three inches deep, and the unsound material removed. Upon removal, the joint should be formed and the material replaced with concrete mix.

Joint sealant is missing in most of the PCC features. It is essential to extend the pavement life. Recommend all the joints be cleaned and sealed following the spall repairs.

B. PISCO AIR BASE

The Pisco airfield consists primarily of a 10,000 ft long runway and a parallel taxiway. The parallel taxiway adjoins the runway via 5 ladder taxiways. One other taxiway is adjacent to the PCC parking apron. Runway 03/21 and all the flexible pavements taxiways can be considered three-layer pavement systems. SATs were conducted every 1000 feet on the taxiway, and runway, on both the base course and subgrade, where possible. This was done to define the soil strength profile. Subgrade tests indicated similar materials and strengths throughout the airfield. SATs were also conducted in random spots throughout the main parking apron.

Pavement conditions at Pisco range from FAILED to EXCELLENT condition. The PCC parking apron is generally VERY POOR and the AC runway is VERY GOOD-to-EXCELLENT. The remaining PCC and AC taxiways vary in condition. Joint sealant is virtually non-existent throughout all PCC features. This has led to edge spalls that present a FOD hazard. The joint sealant that does exist is a sand asphalt mixture. This is brittle and popping out in many areas. It also lends itself to incompressibles penetrating the joints.

AGL calculations indicate load limitations should be imposed on some of the Pisco pavements. Although the runway is in VERY GOOD condition, it is because it has not been subjected to frequent large aircraft loadings. Specific load carrying capabilities for each feature are outlined in the AGL tables, Appendix F. Specific conditions and recommendations are addressed in the following paragraphs.

1. Runway 03/21

Original construction of Runway 03/21 was approximately 8000 feet long with a PCC touchdown on the 21 end. The length of the existing runway is nearly 10,000 feet because of a 2000 ft addition on the 21 end. The first 1000 feet of the Runway 21 is 12 inches of PCC, followed by 1000 feet of flexible pavement. The original PCC touchdown has since been overlayed with 4.5 inches of AC. The remaining 7000 feet of runway is a three-layer flexible pavement. The profile, which was investigated and found to be fairly consistent for all flexible pavements, was evaluated as 18 inches of base course covering the subgrade. Surface thicknesses are based on the actual cores extracted throughout the airfield. SATs were conducted at points throughout the airfield and strengths were found to be fairly consistent. For evaluation purposes, subgrade CBRs equal 25%. Base course CBRs are based on SATs conducted in respective pavement features.

The strength of each layer was determined throughout the runway. The base course strengths generally tested between 30% and 50%. "Averages" were then assigned to the different features. Features were distinguished based on surface course thickness, pavement type, traffic area, and subsurface soil strength. The subgrade strengths were consistent at approximately 25%. The base course and subgrade are gravels and sands with large cobbles. Seashells are abundant in each layer.

Flexible runway pavements are generally VERY GOOD to EXCELLENT condition. Original construction was approximately two inches of AC followed by a 2-4 inch overlay to strengthen the surface. There are no indications of structural distress, and only limited environmentally-related distresses. As was mentioned before, the condition can be highly attributed to this area having only light aircraft landings and the low frequency of loads.

The PCC touchdown, located on the first 1000 feet of the 21 end, is in VERY GOOD condition. Only surface map cracks and a few low severity transverse cracks are apparent. However, as in many of the PCC pavements, the joint sealant is a sand asphalt mix. Recommend the joint condition be monitored and the sealant replaced with a hot-poured asphalt sealant.

2. Taxiways:

There are five ladder taxiways that connect the Parallel Taxiway to the runway. Two are PCC and the remaining are flexible pavements. The conditions range from FAIR to EXCELLENT.

The PCC taxiway sections are generally in GOOD condition with the exception of two, which are in VERY POOR and FAILED condition. One is a small section of PCC near the intersection of Taxiway 2 and the Parallel Taxiway (Feature TlOA). In this section, nearly all the PCC slabs have shattered due to overloading the pavement. The PCC thickness is between seven and eight inches and supporting soils are relatively weak. Recommend this area be replaced. The second area is the PCC (part of Feature AO3B) connecting the main apron to the adjacent AC taxiway. It is in VERY POOR condition. Again, shattered slabs, spalled joints, and intersecting cracks are typical throughout. Recommend this area be replaced.

The other significant taxiway distresses are limited to the AC at the intersection of the Parallel Taxiway and the main parking apron (the south end of Feature T04A). Only 2 inches of AC protects the base course in this area. Isolated depressions and alligator cracking are evidence of pavement failure. Recommend the general area (approximately 800 square feet) be structurally repaired by enhancing the supporting soils and replacing the AC surface course.

3. Aprons:

The primary apron is located on the West side of the parallel taxiway and is constructed of PCC. Different thicknesses indicate different pavement features. Common to all features is the lack of joint sealant. Recommend all joints be cleaned and sealed with a hot-poured liquid asphalt.

The North half of the apron is in FAIR condition with most distresses being low severity intersecting cracks, lack of The south half of the apron joint sealant, and edge spalls. is in POOR, or worse condition. The slab thicknesses in this section range from five to eight inches. The five-inch pavement has failed. The rest of this area is in POOR condition. Shattered slabs, edge spalls, and surface scaling are common throughout. The entire section of apron will eventually require replacement. The most severe areas are where the aircraft travel to and from the parking spots. Shattered slabs are common. If it cannot be done under one contract, recommend the inbound and outbound traffic lanes be replaced first, followed by the remainder of the apron. Recommend the AGL tables be consulted prior to using the apron.

SECTION V: CONCLUSIONS/RECOMMENDATIONS

1. LAJOYA AIR BASE

- a. Joint sealant, where evident, is not properly functioning on virtually all PCC features. Recommend a major joint sealant replacement project be implemented.
- b. Spalled joints are quite common. Recommend the severely spalled areas be repaired.
- c. Diagonal cracks appear in the same general station on both the runway and Parallel Taxiway. Recommend the cracks be sealed.
- d. The PCC cracks on the 17 end of the runway were repaired with rigid material after the concrete was chipped to sound material, resulting in a groove. Recommend these cracks be sawcut to establish a clean, vertical edge, and replaced with rigid material.
- e. AGLs indicate no significant load restrictions on the tested pavements at LaJoya Air Base.

2. PISCO AIR BASE

- a. Many PCC apron features are in POOR, or worse, condition. These pavements should be replaced.
- b. Joint sealant is also in poor condition in many PCC pavements. Recommend a major joint sealant replacement project be implemented. Recommend the sealant be a hot-pour asphalt sealant.
- c. Most PCC slabs on Feature T10A are shattered. Recommend this feature be replaced.
- d. The south area of Feature T4A is structurally distressed. Recommend this area, approximately 800 square feet, be rebuilt.
- e. Significant load restrictions should be placed on many of the pavement features at Pisco Air Base. Based on the AGL calculations, the weakest pavements include Features T04A, A01B, and A02B. Recommend these pavements be structurally enhanced, and/or replaced, and the AGL tables consulted prior to loading these pavements.

SECTION VI: GLOSSARY

Allowable Gross Load (AGL) - The maximum aircraft load that can be supported by a pavement feature for a particular number of passes.

Base or Subbase Courses - Natural or processed materials placed on the subgrade beneath the pavement.

<u>Compacted Subgrade</u> - The upper part of the subgrade, which is compacted to a density greater than the portion of the subgrade below.

<u>Feature</u> - A unique portion of the airfield pavement distinguished by traffic area, pavement type, pavement surface thickness and strength, soil layer thicknesses and strengths, construction period, and surface condition.

<u>Frost Evaluation</u> - Pavement evaluation during the frost-melting period, when the pavement load-carrying capacity will be reduced unless protection has been provided against detrimental frost action in underlying soils.

<u>Pass</u> - On a runway, the movement of an aircraft over an imaginary line 500 feet down from the approach end. On a taxiway, the movement of an aircraft over an imaginary line connecting an apron with the runway. AFR 93-5, Chapter 2.

<u>Pass Intensity Levels (PIL)</u> - Specific repetitions of aircraft over a pavement feature, regardless of time, that are dependent on aircraft design category. AFR 93-5, Chapter 2.

<u>Pavement Condition Index (PCI)</u> - A numerical indicator between 0 and 100 that reflects the structural integrity and surface operational condition of the pavement. AFR 93-5, Chapter 3.

<u>Primary Pavements</u> - Those features that are absolutely necessary for mission aircraft operations. AFR 93-5, Chapter 4.

<u>Subgrade</u> - The natural soil in-place, or fill material, upon which a pavement, base, or subbase course is constructed.

Type A Traffic Areas - Type A Traffic Areas are those pavement facilities that receive the channelized traffic and full design weight of the aircraft. AFM 88-6, Chapter 1.

Type B Traffic Areas - Type B Traffic Areas are considered to be those areas where traffic is more nearly uniform over the full width of the pavement facility, but which receive the full design weight of the aircraft. AFM 88-6, Chapter 1.

Type C Traffic Areas - Type C Traffic Areas are considered to be those on which the volume of traffic is low or the applied weight of the operating aircraft is less than the design weight. AFM 88-6, Chapter 1.

PAVEMENT CONDITION EVALUATION TERMINOLOGY

CONDITION RATING	DEFINITION
EXCELLENT	PAVEMENT HAS MINOR OR NO DISTRESS AND WILL REQUIRE ONLY ROUTINE MAINTENANCE.
VERY GOOD	PAVEMENT HAS SCATTERED LOW SEVERITY DISTRESSES WHICH SHOULD NEED ONLY ROUTINE MAINTENANCE.
GOOD	PAVEMENT HAS A COMBINATION OF GENERALLY LOW AND MEDIUM SEVERITY DISTRESSES. MAINTENANCE AND REPAIR NEEDS SHOULD BE ROUTINE TO MAJOR IN THE NEAR-TERM.
FAIR	PAVEMENT HAS LOW, MEDIUM, AND HIGH SEVERITY DISTRESSES WHICH PROBABLY CAUSE SOME OPERATIONAL PROBLEMS. MAINTENANCE AND REPAIR NEEDS SHOULD RANGE FROM ROUTINE TO RECONSTRUCTION IN THE NEAR-TERM.
POOR	PAVEMENT HAS PREDOMINANTLY MEDIUM AND HIGH SEVERITY DISTRESSES CAUSING CONSIDERABLE MAINTENANCE AND OPERATIONAL PROBLEMS. NEAR-TERM MAINTENANCE AND REPAIR NEEDS WILL BE INTENSIVE.
VERY POOR	PAVEMENT HAS MAINLY HIGH SEVERITY DISTRESSES WHICH CAUSE OPERATIONAL RESTRICTIONS. REPAIR NEEDS ARE IMMEDIATE.
FAILED	PAVEMENT DETERIORATION HAS PROGRESSED TO THE POINT THAT SAFE AIRCRAFT OPERATIONS ARE NO LONGER POSSIBLE. COMPLETE RECONSTRUCTION IS REQUIRED.

SECTION VII: CONVERSION FACTORS

BRITISH TO INTERNATIONAL SYSTEMS (SI) OF UNITS

British units of measurements are used in this report and can be converted to SI (Metric) units as follows:

TO CONVERT	<u>TO</u>	MULTIPLY BY
LENGTH inch (in) inch (in) foot (ft) yard (yd) mile (mi)	<pre>millimetre (mm) metre (m) metre (m) metre (m) kilometre (km)</pre>	25.400 0.0254 0.305 0.915 1.609
AREA square inch (in ²) square inch (in ²) square foot (ft ²) square yard (yd ²) square mile (mi ²) acres	square millimetre (mm ²) square metre (m ²) square metre (m ²) square metre (m ²) square kilometres (km ²) square kilometres (km ²)	645.2 0.0006452 0.093 0.8361 2.59 0.004046
VOLUME cubic inch (in ³) cubic foot (ft ³) cubic yard (yd ³)	cubic millimetre (mm ³) cubic metre (m ³) cubic metre (m ³)	16487.0 0.028 0.7646
MASS pound (1b)	kilogram (kg)	0.454
FORCE pound (1b f) kip (1000 lb f)	newton (n) kilogram (kg)	4.448 453.6
STRESS pound per square inc (psi)	h kilo Pascals (kPa)	6.895
MODULUS OF SUBGRADE pounds per square in per inch (psi/in)	REACTION (K-VALUE) ch kilo Pascals per millimetre (kPa/mm)	0.2715
DEGREES degrees Fahrenheit(° (F°-32)	F) degrees Celsius (^O C)	5/9
DENSITY pounds per cubic foo (pounds mass)	t kilogram per cubic meter (kg/m ³)	16.052

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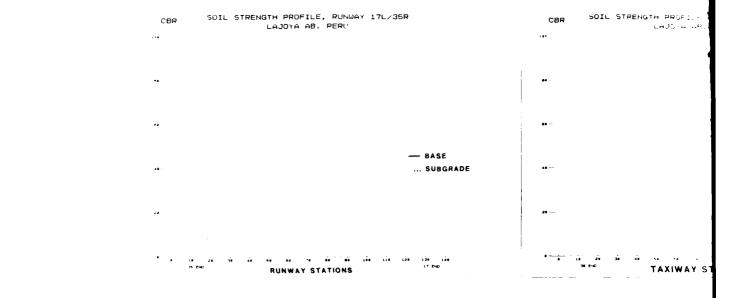
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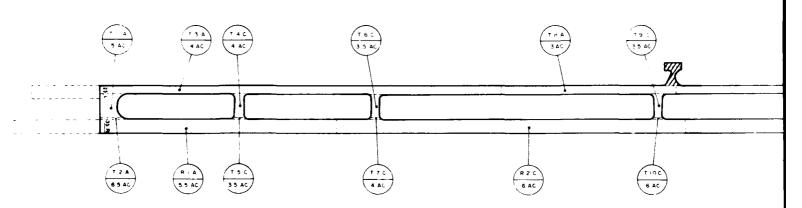
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LAJOYA





<u>LEGEND</u>

FEATURE DESIGNATION SEE NOTE 1, PAVEMENT THICKNESS IN INCHES & TYPE

TYPE OF FEATURE

- R -- RUNWAY T -- TAXIWAY

TYPE TRAFFIC AREA (SEE NOTE 2)

- A A TYPE TRAFFIC
- 8 8 TYPE TRAFFIC
- C C TYPE TRAFFIC
- CHANGE IN FEATURE DESIGNATION
- PCC PORTLAND CEMENT CONCRETE
 AC ASPHALTIC CONCRETE

NOT EVALUATED (N.E)

NOTES

- 1 FEATURE DESIGNATION DENCTE FEATURE FOR GIVEN FEATURE
- 2 TRAFFIC AREA DESIGNATION:
- 3 FEATURE DESIGNATIONS DO NO
- FROM PREVIOUS REPORTS AND

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NOTES

- FEATURE DESIGNATION DENOTES TYPE OF FEATURE, NUMBER OF FEATURE FOR GIVEN FEATURE TYPE AND TYPE TRAFFIC AREA
- 2 TRAFFIC AREA DESIGNATIONS ARE BASED ON AFM 88 6. CHAPTER 1
- 3 FEATURE DESIGNATIONS DO NOT CORRESPOND WITH THOSE FROM PREVIOUS REPORTS AND DRAWINGS

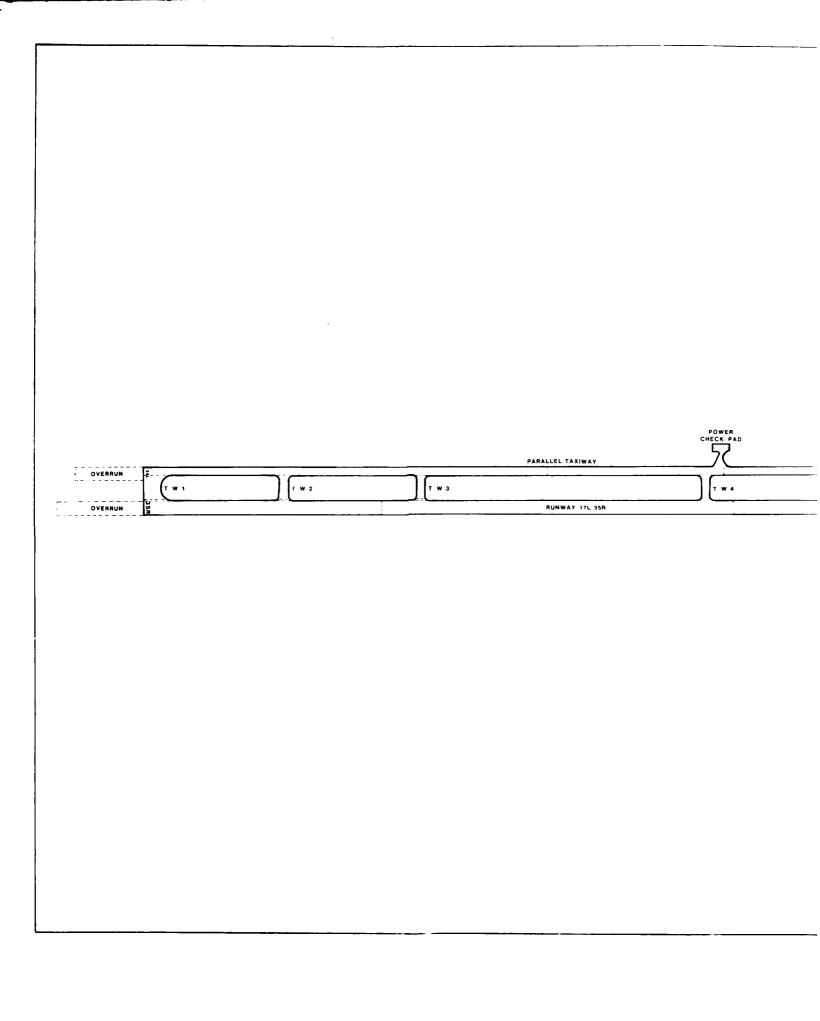


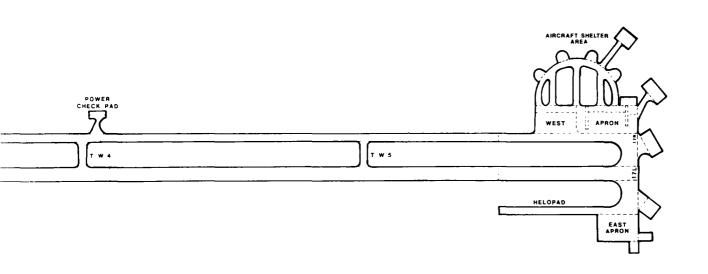


UNITED STATES AIR FORCE ENGINEERING & SERVICES CENTER TYNDALL AIR FORCE BASE, FLORIDA

AIRFIELD LAYOUT PLAN

CHOINEER GABRIELSON	NOV 89	APPENDIX A
SANTIAGO	GRAPHIC	SHEET 1 OF 2



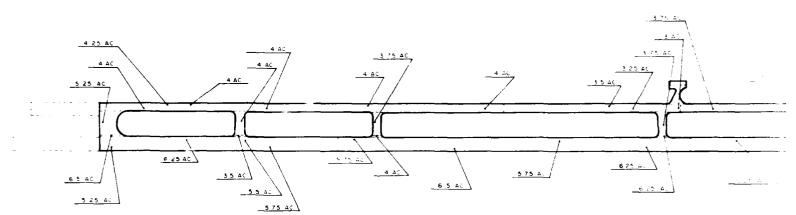


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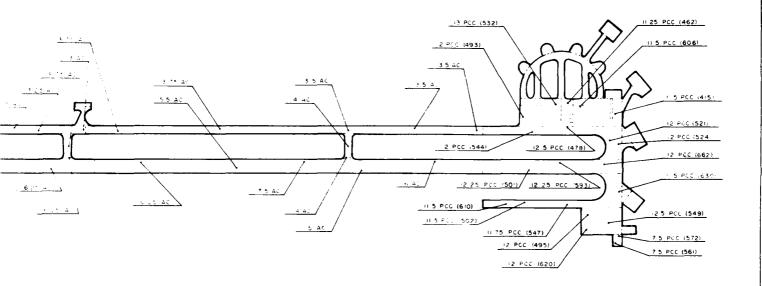
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AIRFIELD DESIGNATIONS

ENGINEER	DATE	
GABRIELSON	NOV 89	APPENDIX A
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LaHUE	GRAPHIC	SHEET 2 OF 2



CORE LOCATION, PA TYPE PAVEMENT, AN CONCRETE FOR PCC



7.5 AC ' 8.5 PCC (578)

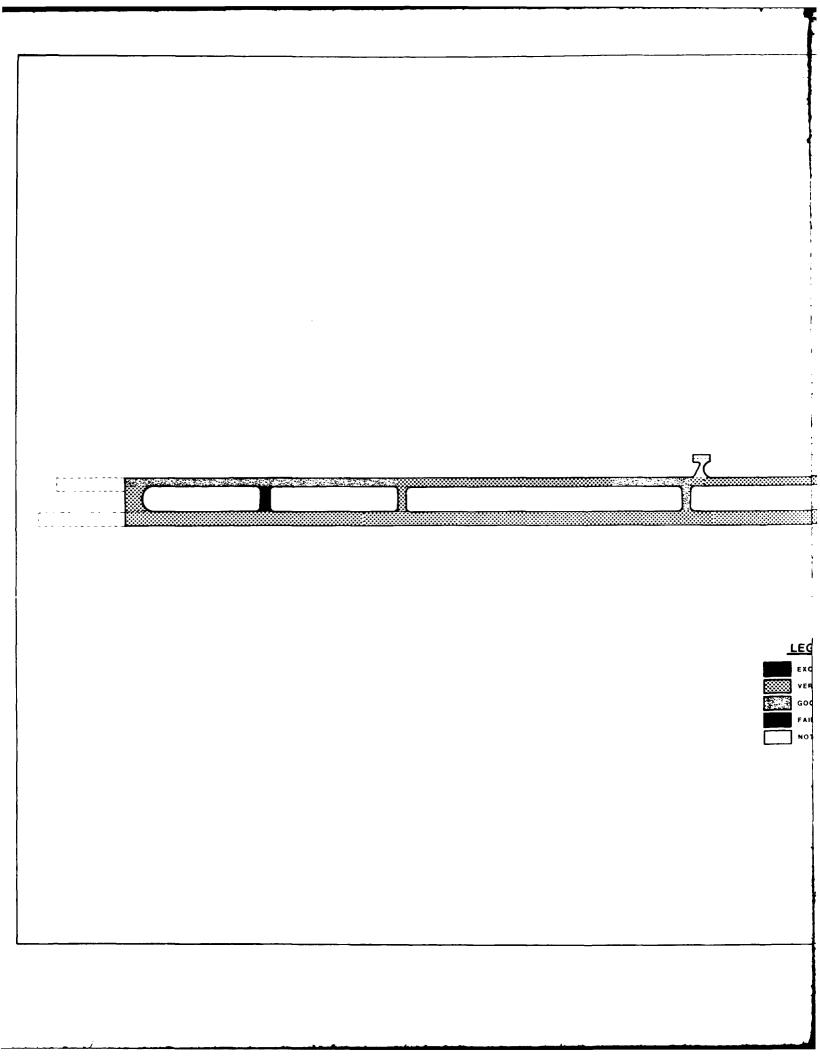
CORE LOCATION, PAVEMENT THICKNESS IN INCHES, TYPE PAVEMENT, AND FLEXURAL STRENGTH OF CONCRETE FOR PCC CORES.

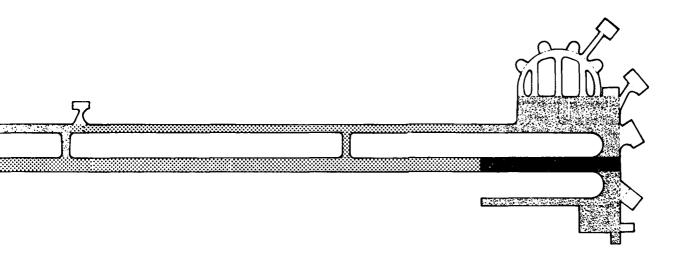


UNITED STATES AIR FORCE ENGINEERING & SERVICES CENTER TYNDALL AIR FORCE BASE, FLORIDA

CORE LOCATIONS

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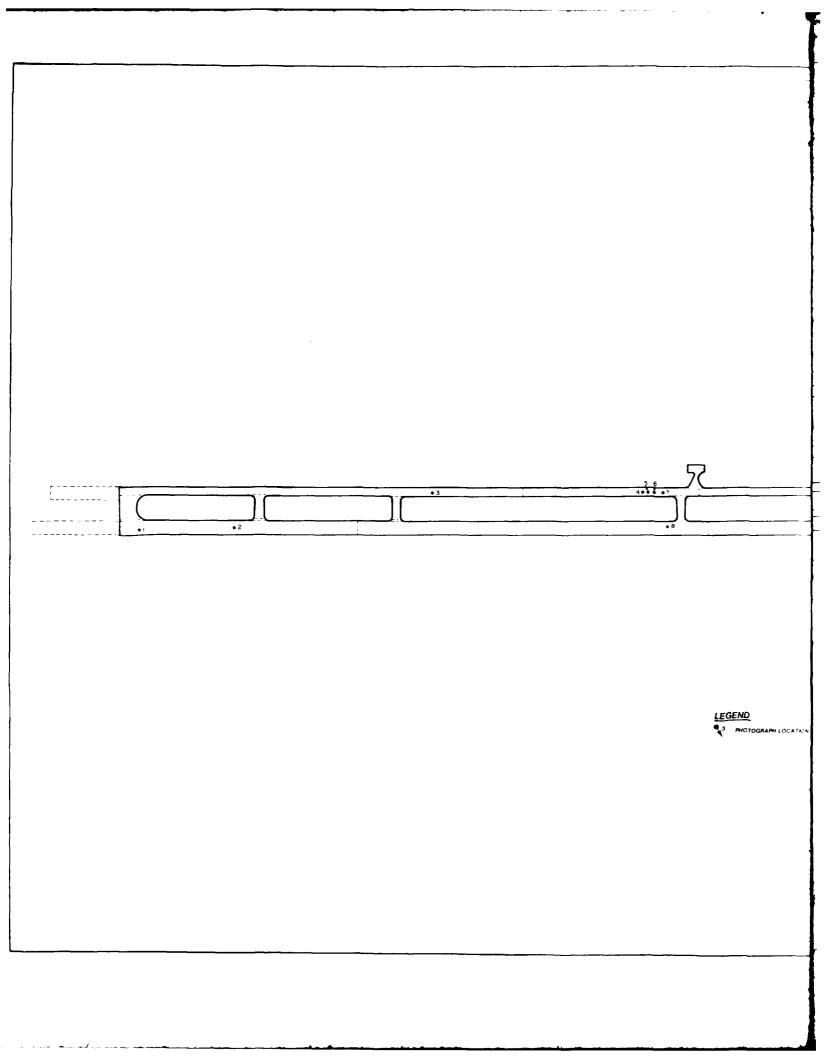
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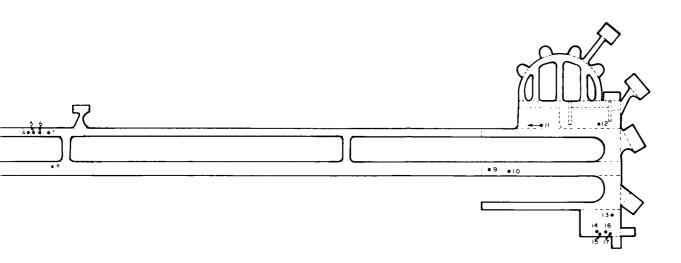


UNITED STATES AIR FORCE ENGINEERING & SERVICES CENTER TYNDALL AIR FORCE BASE, FLORIDA

CONDITION SURVEY

CABRIELSON	NOV 88	APPENDIX O
LaHUE	GRAPHIC	SHEET OF





PHOTOGRAPH LOCATION, DIRECTION, AND NUMBER



UNITED STATES AIR FORCE ENGINEERING & SERVICES CENTER TYNDALL AIR FORCE BASE, FLORIDA

PHOTOGRAPH LOCATIONS

GABRIELSON	NOV B9	APPENDIX D
LaHUE	GRAPHIC	SHEET 2 OF 4



PHOTO 1: Typical patchwork on runway showing excessive asphalt in seal coat and ungulating AC surface.



PHOTO 2: Tire mark in AC patch where asphalt sealant was placed. Excessive asphalt sealant typical in isolated spots on the runway.



PHOTOS 4-7: Diagonal cracks extending across the entire taxiway. Cracks are most likely a result of earth movement and not aircraft loadings. Recommend the cracks be sealed.



PHOTO 5



PHOTO 7

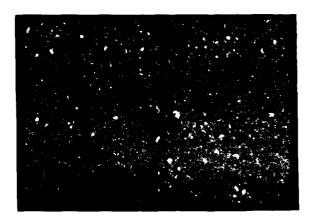
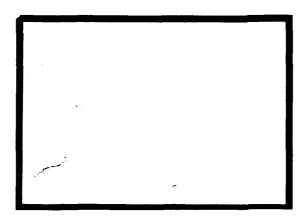


PHOTO 8: Close-up of diagonal runway mack similar to those located on the Parallel Taxiway. Cracks on the runway and taxiway are located in line, indicating subsulface movement.



where essive ted spots on



<u>PHOTO 3</u>: Longitudinal, environmenta., and load-related cracks limited to the outer 3 paving lanes of the Parallel Taxiway. This is common to both sides, but isolated to the Parallel between Taxiways 4 and 5.



PHOTO 6



unway crack Fara..el od taxiway

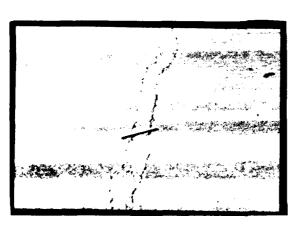


PHOTO 9: "Pepaired" transverse cracks that were chipped to sound material and filled with PCC. Pavement is spalled around the rigid material.

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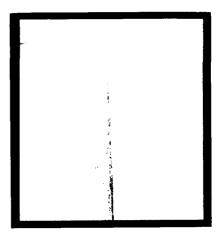
PHOTOGRAPHS

MARIANO MELGAR AIR BASE, LA JOYA PERU

ENGINEER GABRIELSON	NOV 89	APPENDIX D
DRAWN LAHUE	SCALE N/A	SHEET 3 OF 4



PHOTOS 10 & 11: Spalled joints on PCC apron. Joint sealant missiong in most of the PCC. Recommend the distresses be sawcut, material removed, and replaced with new PCC.



PHOTOS 11

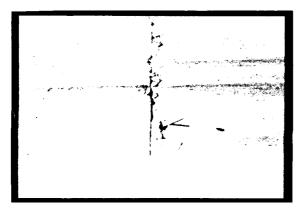


PHOTO 13: High severity joint spall. Joint should be repaired like that recommended in Photos 10 and 11.

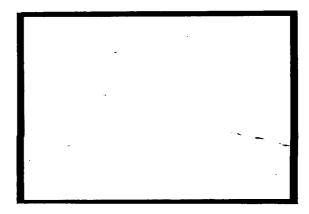


PHOTO 14: Intersecting PCC slab cracks caused from excessive slab dimensions. Recommend the cracks be sealed to minimize moisture and debris infiltrating the pavement.



<u>PHOTO 16</u>: Nonexistent joint sealant typical of many PCC pavements.



PHOTO 17: Evenly spaced cracks extending the length of the apron and isolated to a row of slabs. This type of distress is associated with the vibration and consolidation during construction. Recommend the cracks be cleaned and sealed.

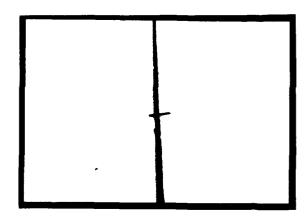


PHOTO 12: Excessively wide joint with no joint sealant.

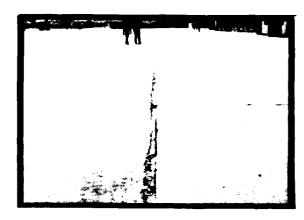


PHOTO 15: Spalled transverse joints which may be a caused from incompressibles prohibiting slab movement resulting in joint spalls.

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extending the to a row of associated ion during iks be



PHOTO 18: LaJoya Air Base pavements.

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TYNDALL AIR FORCE BASE, FLORIDA

PHOTOGRAPHS

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	111	•	•	30		•	•	159	+	345	•	+	† •	302
	IV	+	•	112	+	+	+	<u>+</u>	+	471				441

LAJOYA

SU	MMARY O	FA	LLO	WAB	LE	GRO	รร เ	OAD	S II	N ME	TR	CU	NITS	<u> </u>
FEAT.	PASS INTENSITY				PAVE! FOR	MENT	CAPACI RAFT	TY IN	KILOG	RAMS	x 1000 ERS)		
	LEVEL		2	3	4	5	6	7	8	9	10	11	12	13
	I	+	•	+	+	+	+	+	+	+	•	•	+	+
ROIA	II		•	+	+	+	+		•	+	+	•	. •	•
	III						+			+	•	•	•	•
1	IV		•				+	+	+	•	+	•	+	<u>.</u>
	Ĭ	+	+	+	+	+	•	+	+	+	+	•	•	•
POZC	II	•	+	+	+	+				•		+	•	+
	III	+	+	+	+	+	+	+	+	+	٠	•	+	
ł	IV	+	+	+	+	+	+_	+	•	+		+	· ·	+
	1	+	+	+	+	+	+	+	+	•	+	•	•	•
PO3C	II	+	•	+	+	+	+	+	+	+		+	•	•
	III	+	+	+	+	+	+	+	+	+	+	+	•	
	IV	+	+	+	+	+	+	+	+	+	+	<u> </u>	+	<u> </u>
	I	+	+	48	+	+	+	+		199	•	•		167
RO4A	II	+	+	+	+	+	+	+	•	+	•	+	+	217
	111	+	•	+	+	+	•	+	+	+		•	+	
	IV	+	+	+	+	+	+	+	+	+	+	+	<u> </u>	-
	I	+	+	+	+	+	+	+	+	195	•	•	•	100
TOTA	II	+	+		•	+	+	+	+	207	+	+	•	216
	III	+	•	+	•	+	+	+			+	+	•	•
	IV	+	+	+		+	+	+	+	+	<u> </u>	+		<u> </u>
	I	+	•	+	•	+	+	+	+	•	٠ ا	+	•	+
TOZA	II	+	•	+	•	+	+	+		•	•	•	•	•
	III	+	+	+	+		+	•	+	•	٠	•	•	•
	IV	•	+	+	<u> </u>	+	+	+	+	•	•	•	+	•
	I	+	+	46	+	+	+	+	+	159	•	+	†	150
TOSA	11	•	+	51	•	+	•	+	+	167		+	+	172
	III		+	+	+	•	† •	+	•	184		•	•	יחן
	IV	·	+	+	+	+	+	+	•	213	<u> </u>	<u> </u>		•
_	I	+	+	+	+	+	+	+	+	217	•	•	•	217
TO4C	II	•	+	+		+	+	+	+	+	•	•	•	•
	III	•	+	+	+	+	+	+	+	+	+	•	•	+
	IV	+_	+	+	<u> </u>	+	+	+	<u> </u>	+		<u> </u>	<u> </u>	<u> </u>
	Ĭ	•	•	+	+	•	+	+	+	193	•	•	•	1 80
T05C	II	+	+		•	+	+	+	+	206		+		204
	III	+	+	+	•	+	+	+	•	+	•	•	+	•
	IV	•	+	+	+	+	+	+	<u> </u>	•	•	+	<u> </u>	-
	I	•	•	+	+	+	+	+	+	193	٠	+	+	1 89
₹06C	II	•		•	+	+	+	•	•	206	+	+		205
j	III	+	•	+	•	+	•	•	•	**	•	•	•	•
	IV	+	+	+	<u>+</u>	+'	/ +	+		•	+	•	•	<u> </u>
	ı	• "	•	+	+	•	. +	+	•	217	•	•	+	213
T07C	II	•	•	+	+	+	•	+	+	+	+	•	+	+
}	III	•	•	+	*	•	•	•	•	+	+	•	+	•
	IV		+	+	•		<u> </u>		+	•	+		٠	+
	I	•	25	39	+	+	67	+	142	133	•	227	286	131
TOBA	11	•	28	43	+	•	•	+	+	134	•	243	304	14!
	III	•	•	46	+		•	+	•	150	•		3 34	159
	IA	•	+	52			•	+	•	170	+	•	•	1 44

LAJOYA

SU	MMARY O	FΑ	LLO	WAB	LE	GRO	SS I	OAC	S II	IM V	ETR	IC U	NIT	S
FEAT.	PASS Intensity				PAVE	MENT (CAPAC	ITY IN GROUP	KILOG	RAMS NUME	x 1000 BERS)		
}	LEVEL	1	2	3	4	5	6	7	8	9	10	11	12	13
	I	+	+	+	+	1	+	+	+	204	•	+	+	100
T090	II	•				1 .				217				217
	III					1 .								
ļ	IV													
	Ī	+				1	+	+	+	+	+	+	+	+
TIOC	11				+									+
	III													
	IV						+							
	1	+	•	48	+	+	+	+	+	167		•	 	166
TIIA	11			54				1		177	١.		l .	1 72
1144	III			,		+				194				207
		1			į	1			;	•				
	<u> IV</u>	+	<u> </u>	+	+	 	-	+	-	-	-	·	 	+ +
	1	*		+	•	*		1			:			
T12C	11		•	*				•		l		}		
1	III	•	•	+	•	•	*	•		•			1	1
	IV	<u> </u>	<u> </u>	•	<u> </u>	+	*	+	<u> </u>	•		•		<u> </u>
	I	•	•	+		•	+	•	•	•	•	+	•	+
T13C	II	•	•	•	•	+	•	+	•	•	•	•	•	1 +
	III	•		•	•	•	•	+	•	•	•	+	•	•
	IV	<u> </u>	<u> </u>	•	+	+	+	+	+	<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>
	I	+	+	39		+		74	•	154		+	+	132
T14A	II	•		50	+	•	+	•	•	183	•	•	•	168
	III			+	+	+	+	+	+	+	٠ .	+	+	+
-	IV			•			+	+	•	+		+	•	+
	I	+	+	3 9	•	+	+	74	+	150	•	+	+	132
T15A	II	•		50	•	•		•	•	195		+		167
	III	+	+	+		•		+		•		+		•
	10		•	•			.			•		•		
	1	•	•	37	•	•	65	60	+	145		+	+	116
AOIB	ı.			48						165				143
7010	III			•	•					202				162
	IV	•					+				٠.			
	1	-	•	36	•	-	64	6.0	+	143		+	+	114
A028	II	·		47		;		+	•	163				142
AUZB				*						202	,			187
	111	ļ		ĺ	ļ	1				+				
	IV	+	+	+	*	+		60	144	128		263	3 5 3	101
	I	•	29	31	•	+	57	1	ŀ	į.			+	126
A038	II	•	*	41	*	•	*	72	•	147	(•	ł	!
ļ	111	•	*	50	•	*	+	+	+	1 42				140
	IV	+	-	+	+	 _ +	+	+		+	+	+	+	+
	I	•	•	45	+	+	+	+	•	1 00	•	•	+	144
AO4B	II	•	•	+	*	+	+	+	+	200	*	•	+	184
	III	•		+	•	+	+	+	+	+		•	+	+
	IV	+	•	+	*	+	•	+	+	+	•	+	+	+
	I	25	19	22	•	39	42	45	120	108	274	220	2 96	٨
AO5B	II	•	26	29	•	48	51	54	140	125	322	•	+	00
	111	 •		36	•		+	72	•	156	+	+	+	137
	IV	١.		50	•		+			717				200

LAJOYA F-4

PAVEMENT CLASSIFICATION NUMBERS (PCN) BASED ON 50,000 PASSES OF GROUP INDEX 9 AIRCRAFT

LAJOYA AIR BASE PERU

PPAMIDO	Dest
FEATURE R01A	PCN 104/F/A/X/T
R02C	162/F/A/X/T
R03C	162/F/A/X/T
R04A	100/R/A/X/T
T01A	88/F/A/X/T
T02A	119/F/A/X/T
T03A	70/F/A/X/T
T04C	99/F/A/X/T
T05C	87/F/A/X/T
T06C	87/F/A/X/T
T07C	99/F/A/X/T
T08A	56/F/A/X/T
T09C	92/F/A/X/T
T10C	162/F/A/X/T
T11A	74/F/A/X/T
T12C	105/F/A/X/T
T13C	105/F/A/X/T
T14A	77/R/A/X/T
T15A	79/R/A/X/T
A01B	76/R/B/X/T
A02B	75/R/B/X/T
A03B	60/R/A/X/T
A04B	89/R/A/X/T
A05B	50/R/A/X/T

							AIRC	RAFT	GRO	UP IN	DEX						
			Li	GHT LO	DAD			ME	DIUM L	OAD			HE	AVY LO	AD		
l				2	3	4	5	6	7	8	9	10	11	12	18		
			A-37 C-12 C-21 #C-23 T-37	A-7 A-10 F-4 F-5 F-16 F-10X T-33 T-38 T-39 0V-10 C-20	#F-III FB-III	C-130	C-7 ≋C-9 DC9. C-I4O	737 #T-43	*727 C-22	707 * E-3 C-i35 *KC-i35 VC-i37 DC-8 EC-18 A-300 B-767	C-I4I #B-I B-757	C-5	MKC-IO DCIO LIOII C-I7	747 # E-4 VC-25	8-52		
				<u> </u>								• (CONTROL	LING AIF	CRAFT		
				G	ROSS	WEI	GHT	LIMIT	S FO	RAIR	CRAF	T GR	OUPS	_			
ĺ			1	2	3	4	5	EMENT	CARACI	TY IN H	9	10	1 11	12	13		
LOWEST			5	7	49	69	22	61	92	60	150	325	240	334	180		
HIGHEST GROSS	POS	SIBLE	25	81	114	175	121	125	210	400	477	840	590	850	488		
				L		PA	EMENT	CAPAC	ITY IN	KILOGRA	MS x IC	000	.4	<u> </u>	<u> </u>		
LOWEST GROSS			2	3	22	31	31 10 28 42 27 68 147							151	82		
HIGHEST GROSS			11	37	52	79	55	57	95	181	216	361	267	385	221		
			PASS INT							Y LE	VEL			 			
			L	2	3	4	5	6	7	8	9	10	11	12	13		
		I	300,	000 PAS	SES				15,000 PASSES								
]	LEVEL	□	50,	OOO PAS	SES			15,0	OO PASS	SES			3,000 PASSES				
]	Ē	ш	15,	000 PAS	SES	3,000 PASSES								500 PASSES			
		NZ.	3,	000 PAS	SES	500 PASSES								100 PASSES			

NOTES

- M REFERENCE TO THE ALLOWABLE GROSS LOAD (AGL) TABLE:
 - A Genetes lowest possible empty gross weight of any excraft within the group exceeds the AGL of the perement. Perement cannot support aircreft for respective pass intensity level.
 - + Denotes no weight restrictions. AGL of the pavement exceeds the grounded possible gross weight of any elecate in the group.

Pass intensity levels Σ and Σ are used with reduced subgrave strongths to determine the maximum alternate leads during the frest-men period.

UNITED STATES AIR FORCE ENGINEERING & SERVICES CENTER TYNDLL AIR FORCE BASE, FLORIDA

RELATED DATA

ENGINEER	TOATE	DRAWING HUMOER
N/A	NOV 88	APPENDIX &
DRAWN	BEALS	
L. BASTIAN	N/A	SHEET OF

LA JOYA, PERU

TOPOGRAPHY

La Joya is located 24 miles inland from the South Pacific Ocean in a desert environment. Mountains lie 20 miles to the north through south southeast. The elevations range from 12,000 feet in the north to 5,000 feet in the southeast.

VISIBILITY

There are no significant restrictions to visibility.

SEVERE WEATHER

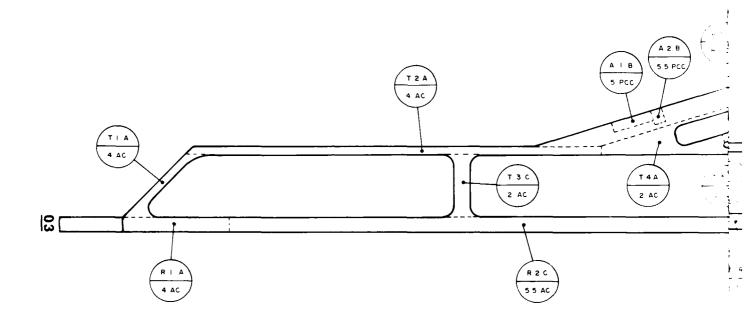
As La Joya is located in southern Peru there is no significant weather. La Joya has a mean annual precipitation rate of less than 10 inches.

APPROVED FOR PUBLIC RELEASE, DISTRIBUTION IS UNLIMITED

7 SORE BEVISION DES ISEE AFR 91 71 LA JOYA PFRU
16° 43' S 071° S-' 'E
ELEVATION 4100 ft
PREPARED RY USAFFTAC
MAR 1989 ≂ EXTREME WIND DATA FOR CONSTRUCTION DESIGN (SEE AFM 88-3, CHAP 1) SNOW 15AD DATA FOR NOOF COMSTRUCTION ISEE AFM 88-3, CHAP 1]
MAXIMUM FROST PENETRATION ISEE AFM 88-3, CHAP 1]
MEAN ANNUAL COOLING DEGREE DAYS 1026 RUNWAYS OR COMBINATIONS FOR CROSSWIND COMPONENT 10 AMOTS OR LESS AIR CONDITIONING DESIGN AND CRITERIA DATA (SEE AFM 88 8, CHAP 6; ANNUAL WIND COVERAGE TABULATION INSTRUMENT RUNWAY |1| WIND COVERAGE ("...) ALL WEATHER |2| WIND COVERAGE ("...) INSTRUMENT MINIER MEATING DESIGN TEMPERATURE ISEE AFM BB B, CHAP 61 PRESSUME ALTITUDE AND TEMPERATURE DATA FOR DETERMINING REQUIRED RUNNAY LENGTHS (SEE AFM 86-2) DATA AVAIL ATBLE MEAN ANNUAL NUMBER OF MEATING DEGREE DAYS 659 ADDITIONAL DATA DATA SHOULD BE SECURED THROUGH THE LOCAL NOTICE WHEN INCESSARY INTERPRETATIONS OF THESE FIELD ELEVATION _4100 ENGINEERING WEATHER DATA MAGNETIC VARIATION SOURCE MEAN WINTER WIND SPEED 2.6. KNOTS 5 THE NOT Y STAFF WEATHER OFFICER MAGNETIC MARK RNO15 ţ AIR FORCE RUKWAY WIND COMPUTER (SEE AFM 86.6) RUMMAY Diagram म्म 77 AMM YRS REC 90 ** 77 222 323 ŧ 0 D INSTRUMENT. CERLING = 200 FEET AND VISIBILITY = 1.2 MM.E. A CELLING SOCIOOFEET AND VISIBILITY = 3 MINES 96.3 D CELLING SOCIOOFEET AND VISIBILITY = 1 MINE OF VISIBILITY = 1 MINE BUT = 3 MINES AND CELLING = 1000° 2.7 FLYING WEATHER ANNUAL PENCENTAGES FOR VARIOUS CATEGORIES 3 AND EITHER CEILING - 1500 FEET OR VISIBILITY - 3 MILES DENOTES LESS THAN 0.5 DAY
DENOTES LESS THAN 0. MINCH 0. 45 INCN 82 0 99 100 2228 Y X X 00 ad C CERLING - 500 FEET AND OR VISIBILITY - 1 MILE **MSTRUMENT** 00 90 SOUTH OF SERVICE SERVI 0 qq 98.7 z Ş 00 00 aa 88 4 311 4, 99 ब्रुव्यन aa 00 90 ٥٥ **#**222 THESE WIND ROSES SHOW THE TOTAL IN OF WINDS BY SPEED GROUP AND EMECTION SASED ON THAT SEASTING. CLIMATOLOGICAL DATA MAM ANNUAL WIND ROSES W54 323X aa 99 #OTE: 822 9 9 aa 90 = 0 aa 9 69 68 69 73 87 25 8 353 99 90 ~ MACE OF DATA NATIONAL INTELLIGENCE SURVEY * 0 **35** E2 00 * ল্লহ * 882 O. 70. INCHES _ 9 ... YEARS OF RECORD . INCHES 9 YEARS OF RECORD 88 ٩ Ž MAXIMUM 24 HOUR PRECIPITATION MAXIMUM 24 MOUR SHOWFALL ALL WEATHER 93.9 **##32**2 dd 7 z Ş RELATIVE NUMBERY (%) MEAN (MCNES) MEAN NO OF DAYS - 0 5 IN DATSAV MAN TEMP = 50 ° F MAN TEMP = 50 ° F MAN TEMP = 32 ° F ا س MEAN BALY BAX MEAN BALY BW LOWEST SHEWFALL MEAN (MICHES) TEMPERATURE PF þ MAM WOW M

USAFETAC FORM 49 MAY 86

PISCO





FEATURE DESIGNATION (SEE NOTE 1) 13 PCC PAVEMENT THICKNESS IN INCHES & TYPE

TYPE OF FEATURE

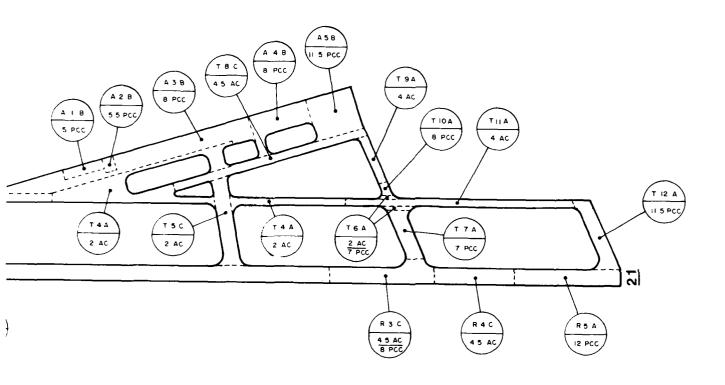
- R RUNWAY
- T TAXIWAY
- A APRON

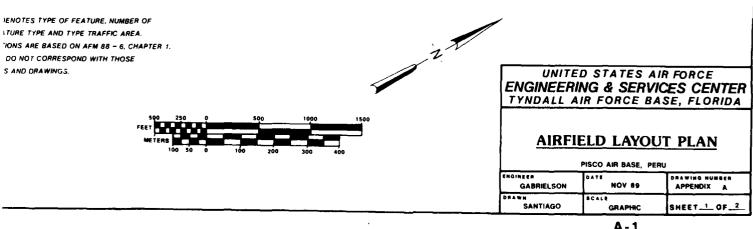
TYPE TRAFFIC AREA (SEE NOTE 2)

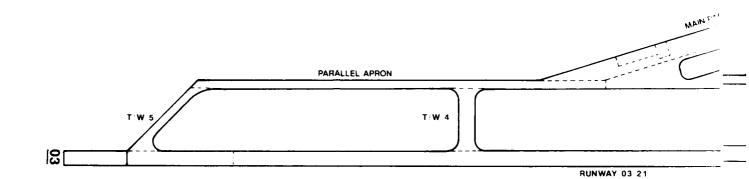
- A A TYPE TRAFFIC
- 8 8 TYPE TRAFFIC
- C C TYPE TRAFFIC
- · · · · CHANGE IN FEATURE DESIGNATION
- PCC PORTLAND CEMENT CONCRETE
- AC ASPHALTIC CONCRETE

NOTES

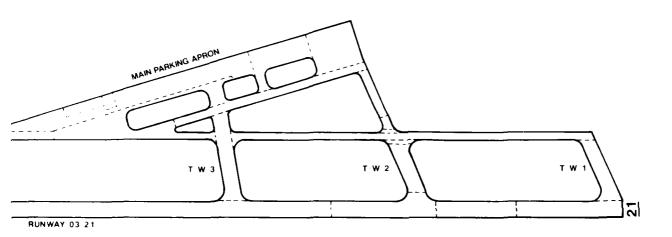
- 1. FEATURE DESIGNATION DENOTES TYPE OF FEATURE, NUMBER OF FEATURE FOR GIVEN FEATURE TYPE AND TYPE TRAFFIC AREA.
- 2. TRAFFIC AREA DESIGNATIONS ARE BASED ON AFM 88 6. CHAPTER
- 3. FEATURE DESIGNATIONS DO NOT CORRESPOND WITH THOSE FROM PREVIOUS REPORTS AND DRAWINGS.

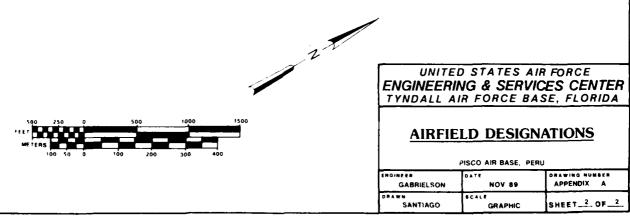


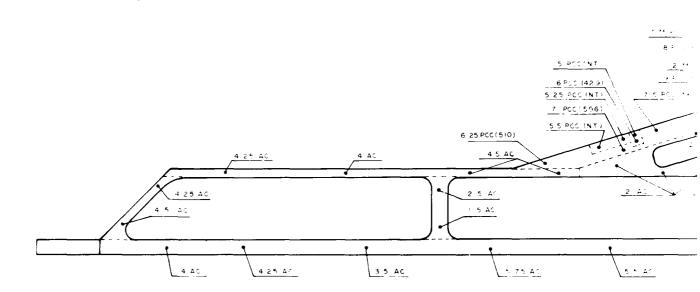




FEET H



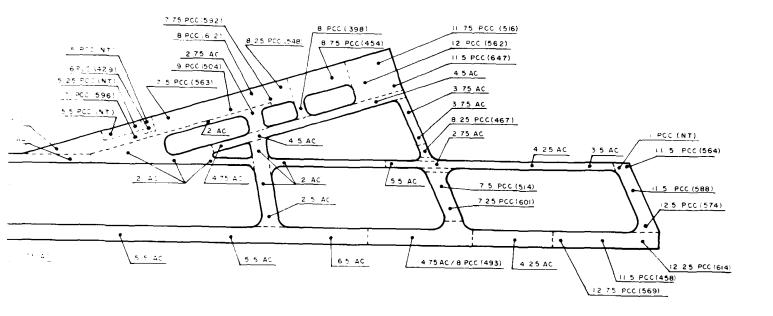




7.5 AC 8.5 PCC (576)

CORE LOCATION. PAVEMENT 1-TYPE PAVEMENT, AND FLEXURA CONCRETE FOR PCC CORES INT: NOT TESTED

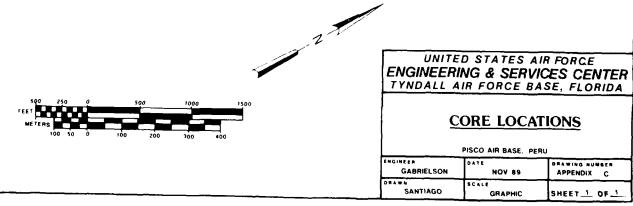


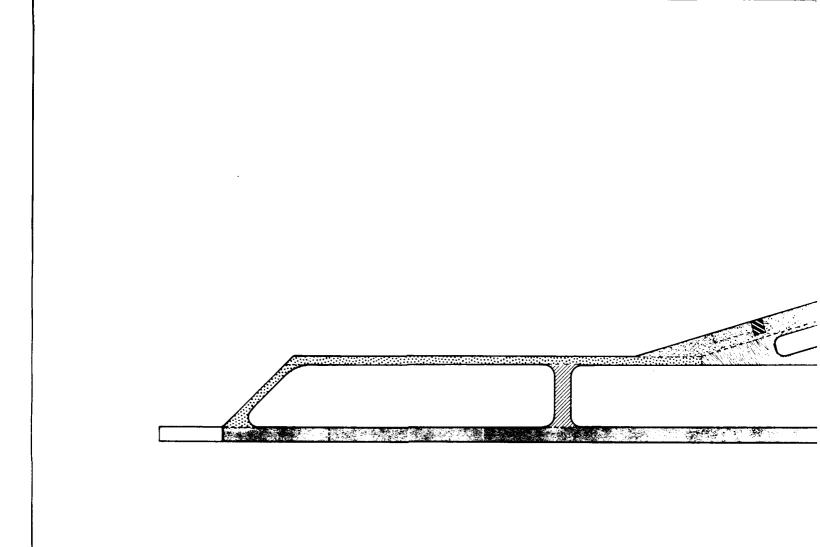


7.5 AC 8.5 PCC (576)

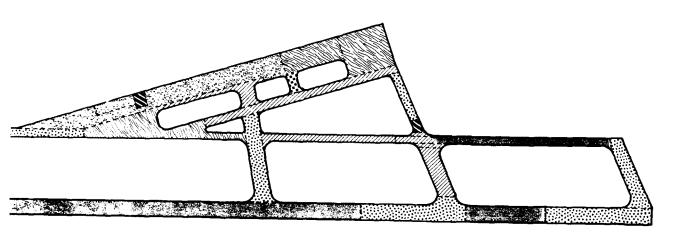
CORE LOCATION. PAVEMENT THICKNESS IN INCHES.
TYPE PAVEMENT. AND FLEXURAL STRENGTH OF
CONCRETE FOR PCC CORES

MT. NOT TESTED





EXCELLENT
VERY GOOD
GOOD
FAIR
POOR
VERY POOR
FAILED
NOT EVALUATED



<u>GEND</u>

CELLENT

RY GOOD

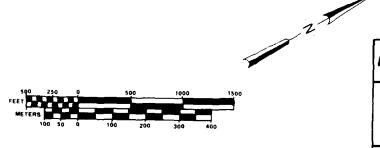
00

OR

RY POOR

LED

T EVALUATED

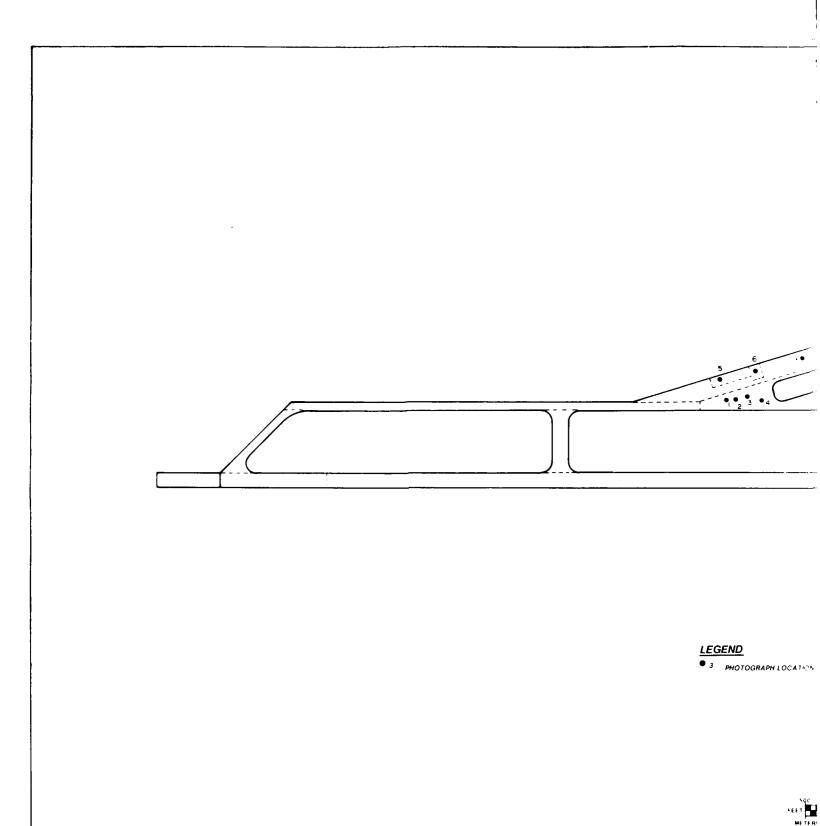


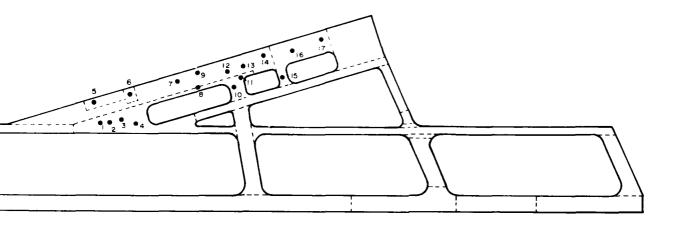
UNITED STATES AIR FORCE ENGINEERING & SERVICES CENTER TYNDALL AIR FORCE BASE, FLORIDA

CONDITION SURVEY

PISCO AIR BASE DERIL

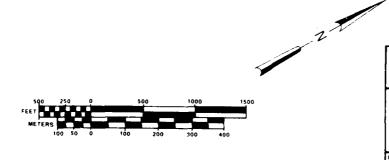
GABRIELSON	NOV 89	APPENDIX D
SANTIAGO	GRAPHIC	SHEET_1 OF 4





<u>LEGEND</u>

• 3 PHOTOGRAPH LOCATION, DIRECTION, AND NUMBER



UNITED STATES AIR FORCE ENGINEERING & SERVICES CENTER TYNDALL AIR FORCE BASE, FLORIDA

PHOTOGRAPH LOCATIONS

PISCO AIR BASE. PERU

ENGINEER	DATE	DRAWING NUMBER
GABRIELSON	NOV 89	APPENDIX D
DRAWN	BCALE	
SANTIAGO	GRAPHIC	SHEET 2 OF 4



PHOTOS 1-4: Pavement overloading is indicated by alligator cracks, block cracks, and depressions. These are common at the intersection of the Parallel Taxiway and the apron. Recommend this area be replaced.



PHOTO 2



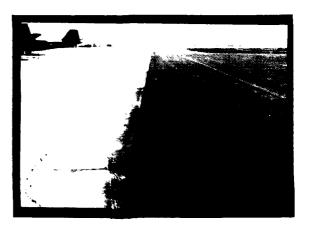
PHOTO 4



PHOTO 5: Severely spalled joint that was patched with AC. The joint should be essuat and repaired with rigid material



PB010 7: Typical shattered slabs concentrated on the apron tax; route. These slabs should be replaced.



 $\underline{P(QTOS,g)}$. Larly signs of pavement farility shown at the intersection of the FCC spin and AC taxibay.



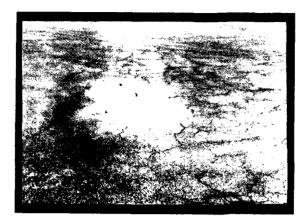


PHOTO 3







 $\frac{PH \times TO - 6}{\text{extensive}}: \quad \text{Failed PCC feature as indicated by extensive shattered slabs}. \quad \text{Recommend the area be replaced}.$



or and to fee upon or the second

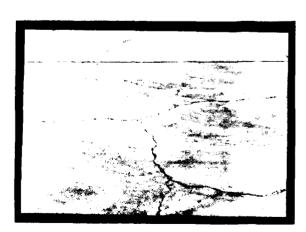


PHOTO 9: Typical shattered pcc s masor apron.

UNITED STATES AIR FORCE ENGINEERING & SERVICES CENTER TYNDALL AIR FORCE BASE. FLORIDA

PHOTOGRAPHS

PISCO AIR BASE. PERU

PISCO AIR BASE. PERO										
ENGINEER GABRIELSON	NOV 89	APPENDIX D								
SANTIAGO	SCALE N A	SHEET 3 OF 4								



<u>PHOTO 10</u>: AC depression which will eventually progress to alligator cracking and pavement failure under aircraft loads.



PHOTO 11: Pothole shown at the intersection of the AC taxiway and the PCC apron.

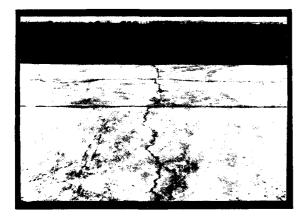
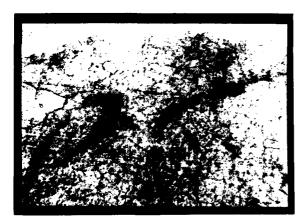


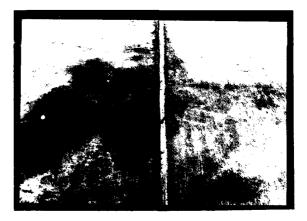
PHOTO 1: Cracks formed where joints should have been cut to control the cracking. Recommend the cracks be cleaned and sealed.



 $\underline{\text{PHOTO}}$ 14: Extremely "tight" transverse joint typical on the apron.



<u>PHOTO 16</u>: Extensive map cracking caused from over-finishing and possible, uncontrolled curing.



 $\underline{PHOTO}(\underline{17})$. Nonexistent joint scalant typical throughout the PCC features.



ie intersection
apron.

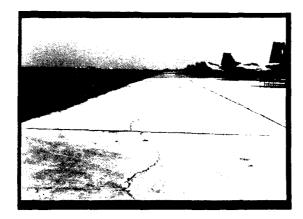


PHOTO 12: Typical longitudinal cracks extending the entire length of the apron. Recommend the cracks be cleaned and sealed.



verse from



PHOTOS 15: Shattered slabs typical accross entire taxiway.





PHOTO 18: Pavement Evaluation Team consisting of (left to right) Capt Jay Gabrielson, Team Chief, SSgt Steve Hadron. Coring Expert, SMSgt Doug Thompson, Consultant, TSgt Ralph Crompton, Team Superintendent, and SSgt Todd Bauder, Soils Expert.

UNITED STATES AIR FORCE
ENGINEERING & SERVICES CENTER
TYNDALL AIR FORCE BASE. FLORIDA

PHOTOGRAPHS

PISCO AIR BASE. PERU

ENGINEER GABRIELSON	NOV 89	APPENDIX D
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PADE		K/CBR	25		25				25				25		22		25		52		25						25
SUBCOAN		DESCRP GP		d5		dg		дъ		d5		д5	, -	GP		д <u>э</u>		6 P		g _p		g _b		d5		ď	
	1000E	ð																									
CHRRACE	300000	DESCRP											•														
	THICK	1																									
	1000	K/CBR	46		9		250		36		250		40		09		40		9		8		350		350		36
BACE	10435	DESCRP		GP		SP		75		35		дS		дĐ		db		СР		GP GP		SP		SP		СР	
RTY DATA	THICK	(18.0		18		18.0		18.0		18.0		18.0		18.0		18.0		18.0		18.0		18.0		18.0		18.0	
PROPE	1000	FEX					200				540												550		550	-	
SUMMARY OF PHYSICAL PROPERTY DATA	AVENCIA!	DESCRP AC		AC		PCC		AC		PCC		AC		AC		AC		ΑC		ΨC		PCC		PCC		AC	
MMARY OF	THICK	10.4		5.5		8.0	,	4.5		12.0		4.0		4.0		2.0	.	2.0		2.0		7.0		7.0		4.5	
1 I.	12	EXE								_		_															
7.7.40	LAY PAVENEN	DESCRP				AC																AC					
	THICK	(in)				4.5																2.0				}	
	100	COND	EXC		 X	VERY 4	 0 0 0		EXC	VERY		VERY	9000	VERY	 a005		1005 1005		FAIR 	VERY	0005	\neg	 900	1 8	 1009	5	0000
	-	(ft)	}	150		150		150		150		7.5		75		150		VARIES		150		50		150		75	
	FACILITY			6170		1000		770		1000		850		3900		909		2600		850		100		200		1900	
	A L	IDENT	03/21 03 END	RUMMAY	03/21	RUNWAY	13/50	RUNMAY	03/21	RUNWAY	03/21	TAXIWAY	so.	PARALLE	TAXIWAY	MAY	4	PARALLEL	TAXIWAY	TAXIWAY	m	TW 2 /	PARALLEL TAXIWAY	TAXIWAY	8	APRON	IAXIWAY
		FEAT		ROZC		R03C		R04C		ROSA	·····	TO1A		T02A		T03C		T04A		T05C		T06A		T07A		708C	

ADE	1000E K/CBR		25				25												
SUBGRADE	DESCRP	d.		<u>6</u> P	_	<u>6</u> b		СР		GР		GP		GP		d9		СР	
	1000E CBR	_				9				-	1	9		9		9		9	
SUBBASE	DESCRP						<u> </u>				•				•		<u>' -</u> -		<u>'</u>
	THICK (in)								-										
	1000E K/CBR		34		300		8		350		210		210		230		250		es S
BASE	DESCRP	d 9		SP		M 9		3 5		SP		SP		SP SP		SP		SP	
	THICK (in)	18.0		18.0	ı	18.0		18.0		18.0		18.0		18.0		18.0		18.0	
	1000E FLEX				460		EXC		570		550		450		550		425		260
PAVEMENT	DESCRP	AC		PCC		AC	•	PCC		PCC		PCC	_	PCC		PCC		PCC	
	THICK (in)	4.0		8.0		4.0		11.5		5.0		5.5		8.0		8.0	_	11.5	
IN	1000E FLEX									<u>.,</u>		-							
AY PAVEMENT	DESCRP																		
OVERL	THICK (in)																	•	
	GEN	VERY	9	I V		ر	באר באר	VERY	9	acca		CATI		9000	¥	0 1 0	<u> </u>	E A 1 B	r r
	WDTH (ft)]		75		75		150		100		100		175		200		400	
FACTI 11Y	LGTH (ft)	450		100		2150		750		400		150		2300		550		450	
FA	IDENI	TAXIWAY	J	T10A TAXIWAY	-	PARALLEL		T12A TAXIWAY	_	AO 18 PARKING	NOW A	A02B PARKING		A03B PARKING		A04B PARKING	NOW LA	PARKING	200
	FEAT	T09A		T10A		T11A		T12A		A01B		A02B		A038		A04B		A05B	-

SUMMARY OF ALLOWABLE GROSS LOADS IN BRITISH UNITS														
FEAT.	PASS INTENSITY				FOR	PAVE AIRCE		CAPAC GROUP			BERS			
	LEVEL	1	2	3	4	5	6	7	8	9	10	- 11	12	13
	I	60	34	A	137	nο	87	103	1 11	170	593	580	3 77	٨
POLA	ΙΪ	•	39	55	148	73	06	112	190	177	570	316	406	114
	III	•	43	59	163	103	109	126	209	197	517	360	452	211
	IV	•	52	68			134	1:2	241	220	54 A	434	547	2:4
	Ţ	+	67	89	+	+	+	+	•	315	•	576	713	300
POZC	II	•	+	102	+	•	•	+	+	340	•	•	778	343
	III	•	•	114	+	+	+	+	+	382	•	•	•	403
	IV	•	+	+	+	<u> </u>	+	+	+	460		+	•	+
Ī	Ĭ	59	47	54	+	70	96	101	260	232	520	465	620	A
RO3C	II	•	60	67		104	110	118	292	259	777	540	7 34	208
1	III	•	+	79	•		137	144	+	306	•		+	262
	IV			100	<u> </u>	<u> </u>	+	+	•	381	•	+	•	336
	Ī	•	42	58	163	105	107	124	215	202	694	344	4 51	108
RO4C	II	•	49	66	•	•	118	136	230	217	697	379	4 80	218
	III	+	56	72		+	137	155	255	241	733	443	5 56	257
	IV	•		86	+_	+	+	+	302	287		557	7 05	312
	I	+	62	67	+	109	116	173	5 00	250	74?	528	713	215
RO5A	ΙΙ	•		84	+	•	135	144	+	290		+	+	250
1	III	+		98	+	•	+	175	+	342		•		374
	IV	+		+	•	+		+		425	٠	+	-	415
	I	.52	29	Α	121	75	76	30	157	149	507	252	3 3(Δ
TOIA	11	57	34	Α .	123	30	83	0.7	166	156	513	274	3.53	Δ
}	111	•	38	51	142	30	95	110	1 22	171	576	313	3 93	103
	IA	•	46	59	164	108	116	132	207	198	571	377	475	221
	I	+	44	64	+	+	114	135	2.35	5.55	750	377	404	220
TOZA	II	•	50	71	•	•	125	146	240	234	759	412	530	241
	III		5 ს	77		+	142	164	273	257	•	470	5 90	275
	IV	•	69	89	•	+	+	+	314	297	+	566	713	332
	I	47	23	٨	11i	58	ρŢ	Δ	120	Δ	322	Δ	Δ	Λ
TOSC	II	50	2.5	Α	116	61	65	۸	135	A	328	Δ	٨	Λ
	III	53	27	٨	123	57	71	٤٦ .	1 44	135	410	240	3.07	A
	IA	59	32	A	137	70	P 4	100	1.61	151	433	49.6	340	^
	I	35	1.5	Α	9.2	43	4.5	٨	۸	Δ	1	Δ	٨	,
TO4A	ŢĪ	37	1.0	Α	85	4.5	47	٨	^	Δ	١	٨	Δ	
İ	III	39	19	A	90	4,0	52	^	104	۸	۸	Λ	Δ	
_	IV	42	22	λ	9.6	5.5	50	Λ	115	٨	1	٨	۸	
	I	47	23	Α	111	59	61	Δ	ני ב	Δ	3 2 3	۸	Δ	Δ
TOSC	II	50	?.5	Λ	116	οl	65	Λ	1.35	٨	334	۸	^	^
	III	53	27	۸	123	57	71	87	144	135	410	240	307	Λ
	IA	59	32	٨	137	78	p 4	100	161	151	437	9.28	3 € 0	^
<u>†</u>	I	43	34	٨	146	67	7:	^	204	105	44·3	172	5.04	Λ
T05A	II	54	44	Α	170	79	٦3	8.0	234	210	551	45]	609	۸
ļ	111	•	53	57		იგ	104	110	2.80	250	592	576	7 PC	211
ļ	IV		•	74	•	•	142	140	+	317				30.
<u> </u>	ŗ	30	31	Λ	134	61	65	Λ	. eu	171	446	341	443	٨
TOTA	11	49	40	Λ	155	71	75	Δ	215	175	513	414	5.62	۸.
ł	III	57	48	52	•	qn	04	100	250	231	625	533	720	201
	17	•	53	96	•		120	115	•	ירי,		•		26

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SUMMARY OF ALLOWABLE GROSS LOADS IN BRITISH UNITS

FEAT.	PASS INTENSITY				FOR	PAVE		CAPAC GROUP			BERS			
	LEVEL	1	2	3	4	5	6	7	8	9	10	11	12	13
	I	T +	4.2	58	163	105	107	124	?15	202	534	344	451	124
TOBC	I I	•	40	66	+		119	176	230	217	577	170	4 6 6	218
	III	•	56	72			137	1. F	2 5 5	241	733	443	554	5.3
	<u> </u>	+	•	9.6	+		+		102	207	+	,57	7.05	312
	I	44	25	A	103	6.4	65	^	1 33	4	431	^	Δ	٨
707A	II	40	29	A	107	60	72	Λ	141	۸	436	233	ين د	٨
	III	5 3	32	A	120	75	50	0.3	154	146	456	265	3 34	Δ
	IV	<u> </u>	30	50	139	71	79	112	179	144	474	321	404	1 bu
	I	Δ	22	Α	91	42	4.5	Α	127	۵	4	230	3 1 1	Δ
TIOA	ΙΙ	Δ	2.8	A	105	49	5.2	Λ	144	Δ.	350	275	369	Δ
	III	40	33	A	127	00	64	^	170	1,51	421	343	464	^
	IV	50	44	^	164	ا من	45	00	212	183	525	437	5.06	^_
	I	+	50	96	+	! + i	+	+		332	! •	566	7 41	3.0
T11A	II	+	•	107	+	+	+	+	+	351	† •	+	•	361
· [III	•	+	116	•	+	+	† •		3.96		†	•	413
ļļ.	<u>I V</u>	+	<u>+</u>	_ + _	•	· ·	· -	ļ		445	ļ .	+	+	
	I	•	+	76	•	+	135	14?	+	302	•	+	•	2-4
T12A	II	,	+	76	+	+	+	168	•	351	•	•	•	311
	III	+	•	115	•	*	•	•	•	4.20	! +	•		405
	IA	+	-	•	+	+	+	+	+	+	· ·	 -	+	•
	I	A	1.7	^	Α	25	٨	Α	^	۵	٨	Δ	Δ	Δ .
A019	II	A .	15	A	^	30	٨	^	^	Λ	1	Δ	Δ .	^
	III	Δ	17	A	75	34	A	^	^	۸	4	Δ	^	^
	<u> </u>	Α	2.2	^	9:	44	40	A .	177	^	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	355	746	Λ
	I	Α	11	A	А	3 ני	Α	٨	Δ	۸ .	Α .	Δ .	Δ .	Λ
A029	ΙΙ	A	: 4	^	Α .	26	Λ	Λ	Δ	۸	1	۸	Δ.	Δ .
	III	A	16	٨	A	32	٨	Λ .		1	1	277	Δ	Δ
	IV	A	$-\frac{21}{1}$	<u> </u>	AP	41	43	<u>``</u>	113	Δ	34)	255	341	Α Λ
	I	A	25	Λ .	105	47	5.2	A	143		345	300	401	,
AG3B	II	40	12	۸	110	5L	6.0	*	1 to 0 1 to 0	143	4,7	363	401	٨
	111	46	3 11	۸	144	60 60	72 96	,,,,,	: (' : 11	107	340	4+1	124	יין "
	I V	εú	40	- 3	112			1 2	ا ــــــــــــــــــــــــــــــــــــ		347	!	·	
	I	Δ	27	^	112	52 40	14	^	177	137	+11	272	3 <i>6</i> 0 431	Λ.
A043 [!!	43	34	^	120	74	71)	į .	203	- · · ·	430	40	0 a'r 4 .⊤	Λ.
. }	III	50	41	A 5.7	155 +	20	104	110	253	274	514	10.	1 57	125
	IV	•	53	70	•	+	123	1.70	3 (5	271	707	C ., K	771	316
4059			5 °+	/ O ภ.ต.	` .		142	1:1	+	3(4)	· ·	7.	+	260
A 0 5 B	III			105					•	351	+	•		277
		*		±05 ! + .	•	•	+			. ,				4 20
	I V	1 * 1												

NOTES

IN REFERENCE TO THE ALLOWABLE GROSS LOAD (AGL) TABLE:

- A Denotes lowest possible empty gross weight of any aircraft within the group exceeds the AGL of the pavement. Pavement cannot support aircraft for respective pass intensity level.
- + Denotes no weight restrictions. AGL of the pavement exceeds the greatest possible gross weight of any aircraft in the group.

The load carrying capacitie: of the pavements reported herein are based on material propertius representative of the in-place conditions at the time this field investigation was conducted.

SU	MMARY O	FA	LLO	WAB									NIT	S
FEAT.	PASS INTENSITY		_	,	FOR	AIRC	RAFT	GROUP	INDEX	RAMS	ERS	,		T=-
	LEVEL	1	2	3	4	5	6	7	8	9	10	11	12	13
	I	27	15	A	63	39	3.0	46	8.5	77	254	131	1.72	^
ROIA	II	+	17	24	67	42	43	50	n6	91	257	143	1.94	я:
	- 111	+	19	26	74	46	49	57	04	90	290	163	2 05	35
	IV	•	23	30		+	60	10	109	107	303	107	249	115
	I	+	30	40	•	+	+	+	•	143	•	243	323	140
RO2C	II		+	46	+	+	+	+		154	+	+	ن وز 3	155
	III		+	51	+			+		173	•		+	* b 5
	IA	+		+	+	+		+		2 C 9	+	<u> </u>	+	•
	I	26	21	24	+	40	43	45	119	105	271	711	281	A
RO3C	ΙĪ		27	30	+	47	40	53	1.32	117	320	240	3 3 3	94
	III			35			62	65		139				118
-	I.A.			45		.	+			172				157
	I	+	10	26	74	47	48	56	97	91	310	156	204	nq
RO4C	11		22	29	;		53	01	104	98	31.6	172	272	90
,,,,,	III	•	25	32	,	;	62	70	115	109	332	201	252	114
ľ	IV			30			02		137	130	,,,,	252	320	141
	I I	+ ·	28	30	•	49	52	55	131	117	336	239	3 2 3	07
2054		1	+	1		, ,	61	65	1	131		+		117
POSA	II	•		38		1	+	70		155				147
ļ	111	1		44	*		ł	i		193				109
	IV	+	+	+	+	•	+	+	+	 		 	 	 -
_	Ī	23	13	۸	5.4	34	34	40	71	67	530	114	149	A
TOIA	11 .	2.5	1.5	Ι Δ	5.7	36	37	44	7 5	7()	232	124	140	A
-	III	+	17	23	54	40	43	40	P?	77	243	142	178	PA
	<u>I V</u>	*	20	26	74	40	5.2	<u>.</u>	74	PO	253	171	215	100
	I	+	19	29	•	†	51	(·1	1.05	100	345	171	2 2 4	97
TOZA	II	•	22	32		+	56	',(112	106	340	197	240	100
	III	+	25	34		+	64	74	123	115	•	?13	2 47	174
	IV		30	40	+	+	+	+	1.42	134		?56	7 2 3	150
	I	21	10	А	50	26	27	Α	57	Δ	170	Ι Δ	Δ .	Δ
T03C	II	22	11	Α	52	27	20	^	61	Δ	190	Δ	Δ	Δ
ĺ	III	24	12	Α	55	30	32	30	<i>t</i> 5	51	135	113	130	
	ΙV	26	14	Α	62	3.5	3.6	45	77	7) 3	175	130	1 63	t.
	I	15	7	٨	3.7	10	20	Λ.	٨	۸	1	Δ	۸	٨
T04A	ΙΙ	16	3	٨	312	20	21	٨	Δ	Δ	1	Δ	Δ	
	III	17	в	Α	40	21	23	٨	40	Δ	۸	Δ	Δ	,
j	IV	10	3	Λ	44	24	26	<i>•</i>	5.2	۸	4	٨	Δ.	Λ
	I	21	10	Α	50	26	27	Α	5.3	Δ	170	Δ	۸	٨
TOSC	II	22	11	Α	5?	27	20	٨	£1	۸	130	Δ .	Δ	Λ
	III	24	: 2	٨	55	30	32	39	7.4	51	114	113	130	٨
	IA	26	: 4	A	62	3.5	30	4.5	73	43	116	130	1.63	۵
	<u>r</u>	19	15	- A	56	30	32	٨	65	0.3	222	169	228	٨
T06A	ΙΙ	24	19	Λ	77	35	37	40	٠ ٥ ۾	35	254	704	276	Δ
	III		24	25	•	44	47	40	127	111	333	261	354	nr
	IV	1 1	+	33		+	64	£ 7		143	, ,,,,	+	+	13-
	I	17				27	29		ne -	77	272	154	210	- i 3
T074		17	14	Α .	60	l		A .	97	n q	232	187		,
TO7A	II	22	18	Α	70	32	34	Λ.				1	255	a, v
-	III	25	21	23	+	30	42	4.5	117	104	211	241	3.26	
	1.4		23	5.0	+	+	24	6.1	•	132		•	•	1.2

SU	MMARY O	FA	LLO	WAB	LE (GROS	SS L	OAD	S II	N ME	ETR	IC U	NIT:	S
FEAT.	PASS INTENSITY				PAVEN	AENT (CAPACI RAFT	TY IN GROUP	KILOG	RAMS NUME	x 1000 BERS)		
İ	LEVEL	1	2	3	4	5	6	7	8	9	10	11	12	13
	I	+	19	26	74	47	48	5€	97	0.5	31 2	150	204	6.3
T08C	II		22	29	+	+	53	(-1	104	34	31.5	172	2.22	0.9
1	III	+	2.5	32		+	02	70	115	100	332	201	256	114
	VI	+	+	39	•	· .	+	+	137	131	•	7. !	3.20	141
	Ĭ	19	11	A	45	20	20	A	40	۸	175	Δ	٨	Δ
T09A	II	22	13	А	49	30	32	٨	64	1	127	105	1.36	Δ
	III	24	14	A	54	34	36	4.2	60	55	237	120	151	Δ
	ΙV	<u> </u>	17	22	63	41	44	50	80	74	274	145	1 93	OR
	I	Α	9	۸	41	19	20	Α	57	٨	١	104	141	Δ
TIOA	ΙΪ	Δ	12	Α	47	22	23	٨	45	Δ	153	124	1 47	^
	III	18	14		57	27	20	٨	77	57	171	155	210	^
	1 /	22	10	`_	74	36	36	40	76	05	550	198	270	Δ
	I	+	29	43	+	+	+	+	+	150	+	256	336	143
TIIA	ΙΙ	+	+	4.9	+	+	+	+	•	159	+	+	•	163
	III	•	+	5.2	+	+	+	+	+	175	+	 	· •	דחן
	IV	+	+	+	+	+	+	+	+	202	+	+		•
	I	+	+	34	+	+	61	6.4	•	130	+	+		115
T124	ΙΙ	+	+	43	+	+	+	76	+	157	+		† *	141
,	111	+	+	5.2	+	+	+	+	•	190	•	+	•	193
	ΙV	+		+	+	+	+	+	+	+	•	+	<u> </u>	*
1	I	Α .	5	^	٨	11	۸	Α	^	^	١ ،	Δ.	Δ	^
A019	ΙΙ	Δ.	б	۸	Δ	12	Λ	٨	^	۸	7	^	^	^
1	III	Δ	7	Λ	34	15	٨	Λ.	Δ	۸	Λ	٨	Δ	Δ
	IV	Δ	?	Α	43	10	. ? C	Α	1 3	^	1	lir	154	٨
1	I	Α	4	Δ	٨	10	Λ	د	Δ	3	Δ	Δ	Δ	Δ
1029	ΙΙ	Δ	U	Α	Α	11	A	Δ	Δ	۸	١	۵	Δ	Δ
1	III	A	7	Α	Λ	14	Д	Λ	٨	Δ	Δ	Λ	۵	۸
	<u>t v</u>	Α	?	٨	30	12	10	٨	53	Δ	1	105	141	۸
	I	Δ	11	۸	47	22	23	Λ	64	٨	154	115	1 54	^
AOSA	II	19	: 4	A	54	25	27	٨	7?	r4	174	1.34	107	^
	III	50	17	A	45	31	33	*	4 5	75	237	167	2.24	^
	IV	26	2.2	24	+	41	43	41.	īvė	<u> </u>	224	210	293	
	Ĭ	Δ	12	Α	50	23	24	٨	F 3	4,9	154	153	1 45	Δ
1049	II .	10	15	Λ	5 3	37	50	۸	73	• •	1,44	140	1.95	Λ
	III	22	13	۸	70	33	3.5	٨	0.5	`1	277	101	243	^
	IV	•	24	25	+	44	۷7	4.0	1.14	17:	,,,	220	3 (1)	(1 Q
	Ĭ	+	29	31	+	+	ς.	5١.	137	ורי	3 % 1	747	3 7 1	ر ن
A058	II	•	•	40	+	•	6.4	((•	130	٠	+	•	119
	III	•	•	47	•		+	+	•	153	٠	•	+	149
	In	+	٠	+	+	•	•	*		2.72	+	<u> </u>	•	190

NOTES

IN REFERENCE TO THE ALLOWABLE GROSS LOAD (AGL) TABLE:

- A Denotes lowest possible empty gross weight of any aircraft within the group exceeds the AGL of the pavement. Pavement cannot support aircraft for respective pass intensity level.
- + Denotes no weight restrictions. AGL of the pavement exceeds the greatest possible gross weight of any aircraft in the group.

The load carrying capacities of the pavements reported herein are based on material properties representative of the in-place conditions at the time this field, investigation was conducted.

PAVEMENT CLASSIFICATION NUMBERS (PCN) BASED ON 50,000 PASSES OF GROUP INDEX 9 AIRCRAFT

PISCO AIR BASE PERU

FEATURE	PCN
RO1A	28/F/A/X/T
R02C	62/F/A/X/T
R03C	52/R/B/X/T
R04C	35/F/A/X/T
R05A	59/R/B/X/T
TOIA	23/F/A/X/T
TO2A	40/F/A/X/T
T03C	17/F/A/X/T
T04A	10/F/A/X/T
T05C	17/F/A/X/T
T06A	39/R/B/X/T
T07A	35/R/B/X/T
T08C	35/F/A/X/T
T09A	18/F/A/X/T
T10A	20/R/B/X/T
T11A	65/F/A/X/T
T12A	72/R/B/X/T
A01B	9/R/B/X/T
A02B	7/R/B/X/T
A03B	24/ /B/X/T
A04B	26/R/B/X/T
A05B	62/R/B/X/T

							AIRC	RAFT	GRO	UP IN	DEX		بربنوسه الله الله		
			LI	SHT LO	DAD			ME	DIUM L	OAD			HE	AVY LO	AD
				2	3	4	5	6	7	8	9	10	11	12	15
			A-37 C-12 C-21 HC-23 T-37	A-7 A-10 F-4 F-5 *F-15 F-16 F-10X T-33 T-38 T-39 OV-10 C-20	#F-III F8-III	C-130	C-7 #C-9 DC9. C-I40	737 *T-43	*727 C-22	707 * E-3 C-i35 *KC-i35 VC-i37 DC-8 EC-18 A-300 B-767	C-141 *B-1 B757	C-5	#KC-IO DCIO LIOII C-I7	747 * E-4 VC-25	·8-52
ł				<u></u>				L					CONTROL	LING AIF	CRAFT
							,			R AIR				12	13
l			1 2 3 4 5 6 7 8 9 10 11 PAVEMENT CAPACITY IN KIPS												
			5	7	49	69	22	61	92	60	150	325	240	334	180
			25	81	114	175	121	125	210	400	477	840	590	850	488
						PA	VEMENT	CAPAC	ITY IN	KILOGRA	MS x 10	000			
			2	3	22	31	10	28	42	27	68	147	109	151	82
			11	37	52	79	55	57	95	181	216	381	267	385	221
							PAS	S INT	ENSIT	Y LE	VEL				
1 .			1	2	3	4	5	6	7	8	9	10	11	12	13
[GROSS WEIGHT HIGHEST POSSIBL			OOO PAS	SSES			50,0	15,000 PASSES						
GROSS WEIGHT HIGHEST POSSIBL GROSS WEIGHT			50,	000 PAS	SSES			15,0	OO PASS	SES			3,0	OO PASSI	ES
	I II		15,	000 PAS	SSES			3,0	OO PAS	SES			5	OO PASSI	ES
, ,		区	3,	000 PAS	SSES			5	OO PAS	ĖES			I t	OO PASS	ES

NOTES

- IN REFERENCE TO THE ALLOWABLE GROSS LOAD (AGL) TABLE:
 - A Denotes lowest possible empty grass weight of any sircraft within the group exceeds the AGL of the pavement. Pavement cannot support alreaft for respective pass intensity level.
 - Denotes no weight restrictions. AGL of the pavement exceeds the greatest possible grass weight of any electric in the group.

Pass intensity levels $\underline{\underline{Y}}$ and $\underline{\underline{Y}}$ are used with reduced subgrace strengths to determine the maximum allowable loads during the frost-melt period.

UNITED STATES AIR FORCE
ENGINEERING & SERVICES CENTER
TYNDLL AIR FORCE E, FLORIDA

RELATED DATA

ENGINEER N/A	NOV 88	APPENDIX G
L. BASTIAN	BCALE N/A	SHEET 1 OF

PISCO, PERU

TOPOGRAPHY

Pisco airport is located on the South Pacific coastline just four miles south of the town of Pisco and is at sealevel. The Bay of Paracas is 3 miles south of the airport. A desert plateau lies five miles to the east through southeast. The town of Lima is 130 miles to the north.

VISIBILITY

Visibility can be expected to be reduced below three miles due to fog on at least 12 days a year with May and June having the most days of three and two respectively. Only three days a year will see visibilities reduced below one mile.

SEVERE WEATHER

With Pisco being located on the eastern Peruvian coast there is no significant weather. The mean annual precipitation rate for Pisco is less than 10 inches.

APPROVED FOR PUBLIC RELEASE, DISTRIBUTION IS UNLIMITED

ANNUAL WIND COVERAGE TABULATION	RUNWAYS OR COMBINATIONS FOR CROSSWIND COMPONENT	AV MACMETIC TOUR LEAGE.	MARK		ы	03-21 2° 30' 9908 0.6 0.1								12) WIND COVERNOE 1.0. ALE WEATHER		ADDITIONAL DATA	FIELD ELEVATION 39 FEET MSL	MAGNETIC VARIATION	SUURCE	14AX				ENGINEERING WEATHER DATA		AIR CONDITIONING DESIGN AND CRITERIA DATA (SEE AFM 88 8 CHAP 6)	MINITER MEATING DESIGN TEMPERATURE ISEE AFM 88 8. CHAP 6. Mean miniter wind coef. ()	MEAN ANNUAL MINESTER OF MEATING DEOFF DAY 747	DE TERMINING	RUNWAY LENGTHS JSEE AFM 86-2]	EXTREME WIND DATA FOR CONSTRUCTION DESIGN ISEE AFM 88-3, CHAP]	MAXIMUM FROST PERETRATION ISEE THE 88 - 3, CHAP I	MEAN ANNUAL COOLING DESKEE DAYS 1509	NOTICE WHEN NECESSARY, INTERPRETATIONS OF THESE	DATA SHOULD BE SECURED THROUGH THE LOCAL	STAFF WEATHER OFFICER		~		RREPARED BY CAPETAC	
	YRS REC	3410	14 DIAGRAM	14	14	14		14	14	5	2		101	2 =	*	14						T		۳]	•	AIR CONDITION	MINIER MEATIN Mean winter ,	PEAN ANNUAL	PRESSURE ALTI	REDUIRED	EXTREME WIND Snow Load da	MAXIMUM FROS	TEAIN ANNU	NOTICE WHEN	DATAS	STAFF			AIR FORCE RUNWAY WIND COMPUTER ISEE AFM RG GI	27 23	*
	ANN YR		102	74	79	33		1	9	0 0	3			10	1	80	1		۸.								_	_	٤٢			E	_	_	35				RCE RUNW	2 D	
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